Real-time Hybrid Simulation (RTHS): Background, Theory and Implementation

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NHERI Lehigh Director

Outline of Presentation

- RTHS Background
- RTHS Developments/Implementation at NHERI Lehigh
- Applications

Background

- Dynamic testing of structures
  - Shake table testing
    - Most realistic method of dynamic testing of structures
    - Limitations:
      - Prototype scaled to accommodate shake table capacity
      - Expensive
  - Hybrid and real-time hybrid simulations (RTHS)
    - Viable alternative to shake table testing
  - Effective force testing
    - Force controlled test and requires all the mass to be present in the lab
    - Limitations:
      - Not economical
      - Force control is more difficult than displacement control

RTHS: Background

- Combines experimental and analytical substructures
  - Experimental substructure(s)
    - Not well understood and modeled analytically
    - Full scale component can be easily accommodated
    - Rate dependent devices (e.g., dampers, base-isolators) can be tested
  - Analytical substructure(s)
    - Well understood and modeled numerically
    - Various substructures possible for a given expt. substructure
    - Damage can accumulate (not a problem) provided it can be modeled
- Advantages
  - Cost effective large-scale testing method
  - Comprehensive system response
  - Meets the need of the earthquake engineering community

RTHS Background

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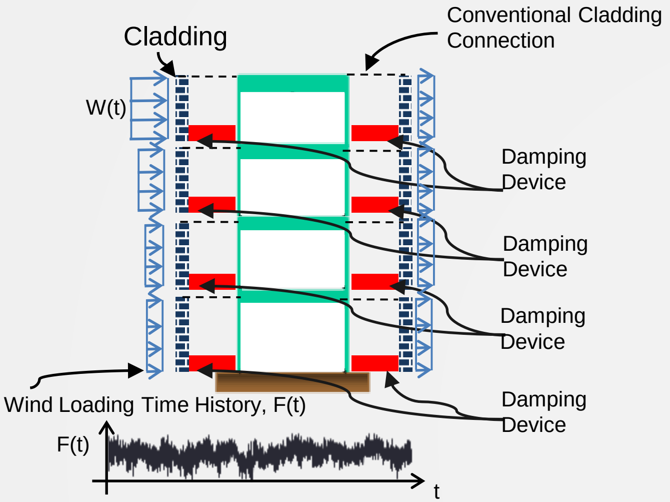
Overall Concept of Real-time Hybrid Simulation : Structural System Subject to Wind Loading

NSF CMMI: Semi-Active Controlled Cladding Panels for Multi-Hazard Resilient Buildings

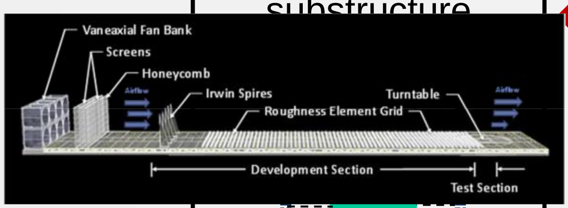
- S. Laflamme (Iowa State), J. Ricles (Lehigh University), S. Quiel (Lehigh University)

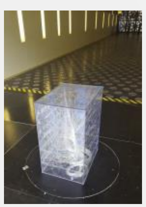


4-Story Building with Semi-active Cladding System

Structural System

Analytical substructure





Wind Tunnel Tests NHERI@UF

Wind Load Determination

(Modeled in the computer)

Real-time input (Forcing Function): Wind Tunnel Data

Cmd Displ $\mathbf{X}_{i+1}^{a}$

Cmd Displ $\mathbf{X}_{i+1}^{e}$

Simulation Coordinator

$$\mathbf{M}\ddot{\mathbf{X}}_{i+1} + \mathbf{C}\dot{\mathbf{X}}_{i+1} + \mathbf{R}_{i+1}^{a} + \mathbf{R}_{i+1}^{e} = \mathbf{F}_{i+1}$$

$\mathbf{R}_{i+1}^{a}$ *Restoring Force*

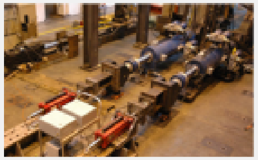
$\mathbf{R}_{i+1}^{e}$ *Restoring Force*

Integrates Eqns of Motion

Real-time structural response

Experimental substructure

(dampers)



(Modeled in the lab)

Why Real-Time Hybrid Simulation?

- **Enables cost-effective large-scale dynamic tests**
  - ➢ Low experimental cost compared to a full shake table test
  - ➢ Various analytical substructures can be used for a given experimental substructure, enabling extensive and comprehensive experimental studies
  - ➢ Meets the need of the natural hazards engineering community of providing experimental validation of concepts for natural hazards mitigation
- **Accounts for rate-dependency of physical specimens**
  - ➢ Rate-dependent structures (frictional devices, dampers, base-isolators, tuned mass damper, etc.) can be investigated with RTHS

  ➔ not possible with conventional, slow hybrid simulation

RTHS: Implementation issues and challenges

Simulation coordinator

- ❑ Numerical integration algorithm
  - Accurate
  - Explicit
  - Unconditionally stable
  - Dissipative

  Preferred
- ❑ Fast communication

Experimental substructure

- ❑ Large capacity hydraulic system and dynamic actuators required
- ❑ Actuator kinematic compensation
- ❑ Robust control of dynamic actuators for large-scale structures

Analytical substructure

- ❑ Fast and accurate state determination procedure for complex structures

RTHS: Implementation issues and challenges

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  - Explicit
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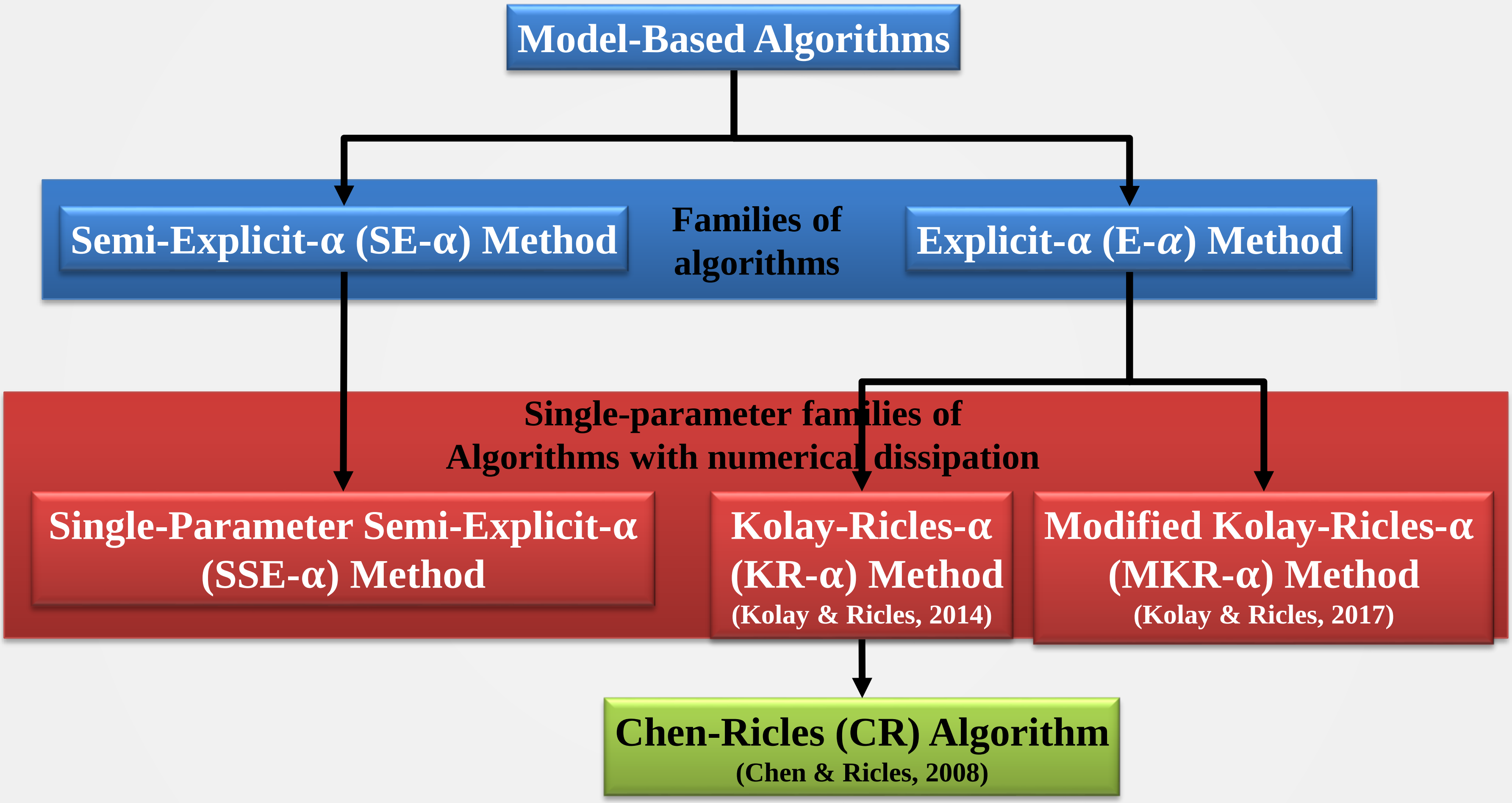
- ❑ Fast communication

NHERI Lehigh Solutions

- Various explicit model-based algorithms
- RTMD real-time integrated control architecture

Model-based explicit algorithms for RTHS

NHERI Lehigh Solutions to RTHS Challenges



Kolay, C., & Ricles, J. M. (2015). Assessment of explicit and semi-explicit classes of model-based algorithms for direct integration in structural dynamics. *International Journal for Numerical Methods in Engineering*. doi:10.1002/nme.5153

Numerical Integration Algorithms

Explicit Modified KR-α (MKR-α) Method

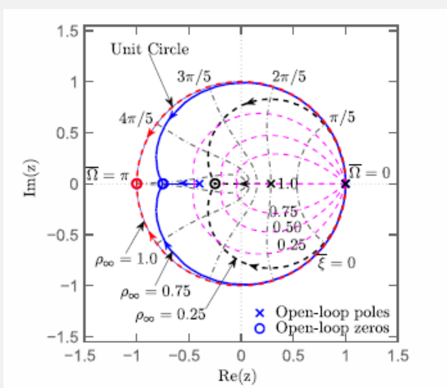
- Explicit Integration of Equations of Motion
- Unconditionally Stable
- Controlled Numerical Damping – eliminate spurious high frequency noise

$\boldsymbol{\alpha_1}$, $\boldsymbol{\alpha_2}$, and $\boldsymbol{\alpha_3}$: model-based integration parameters

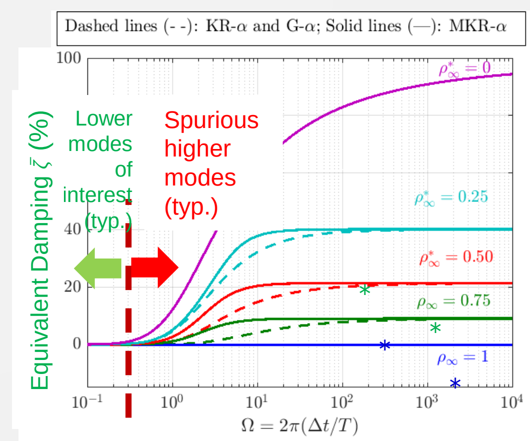
Velocity update: $\dot{\mathbf{X}}_{n+1} = \dot{\mathbf{X}}_n + \Delta t \boldsymbol{\alpha_1} \ddot{\mathbf{X}}_n$

Displacement update: $\mathbf{X}_{n+1} = \mathbf{X}_n + \Delta t \dot{\mathbf{X}}_n + \Delta t^2 \boldsymbol{\alpha_2} \ddot{\mathbf{X}}_n$

Weighted equations of motion: $\mathbf{M}\widehat{\ddot{\mathbf{X}}}_{n+1} + \mathbf{C}\dot{\mathbf{X}}_{n+1-\alpha_f} + \mathbf{K}\mathbf{X}_{n+1-\alpha_f} = \mathbf{F}_{n+1-\alpha_f}$



Stability: Root-Loci



Controlled Numerical Damping

Kolay, C., and J.M. Ricles (2014). Development of a family of unconditionally stable explicit direct integration algorithms with controllable numerical energy dissipation. *Earthquake Engineering and Structural Dynamics*, 43(9), 1361–1380. http://doi.org/10.1002/eqe.2401

Kolay, C., and J.M. Ricles (2017) "Improved Explicit Integration Algorithms for Structural Dynamic Analysis with Unconditional Stability and Controller Numerical Dissipation," *Journal of Earthquake Engineering*, http://dx.doi.org/10.1080/13632469.2017.1326423.

Integration Parameters

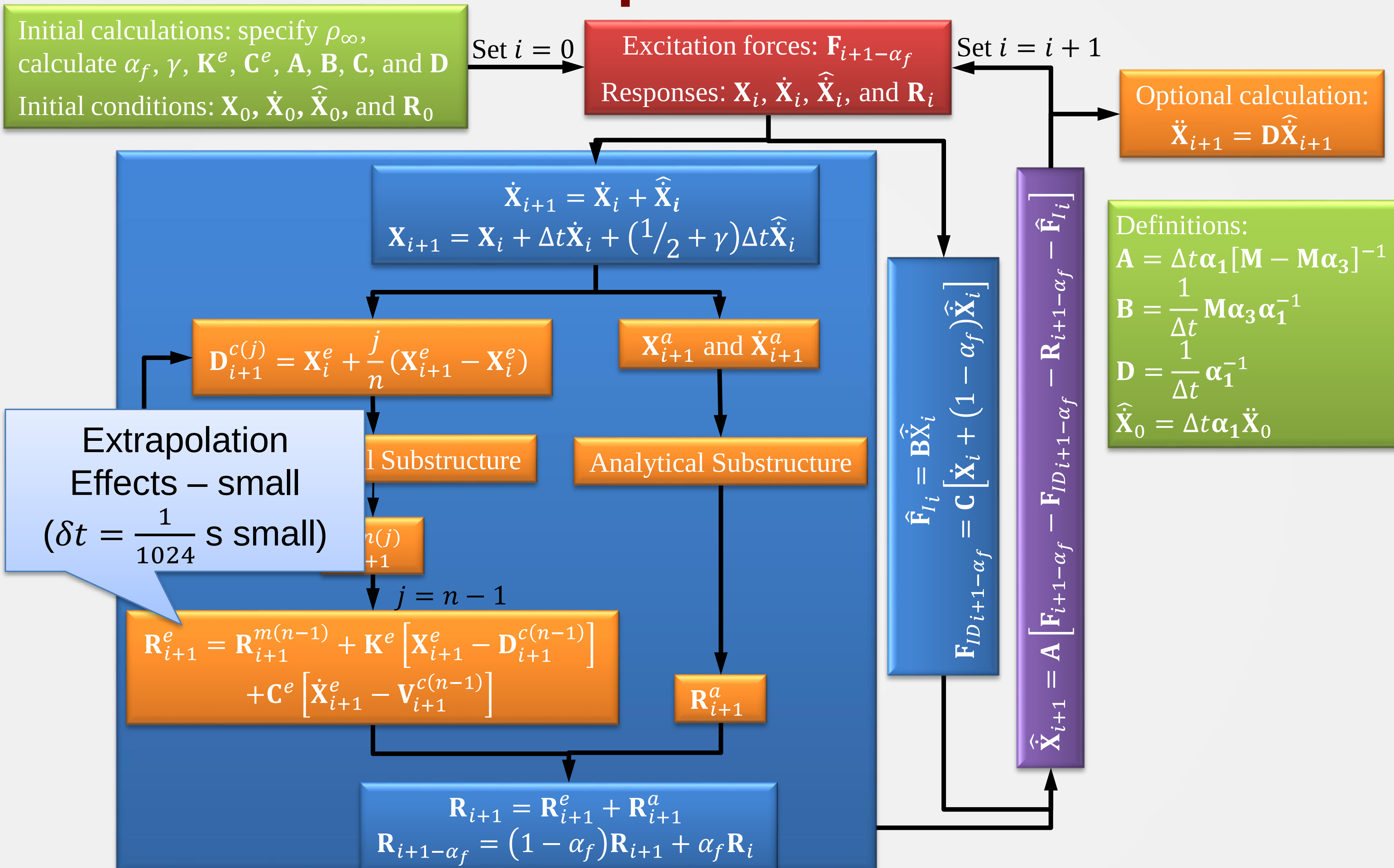
- Parameter controlling numerical energy dissipation
  - ➢ ρ_∞ = spectral radius when $\Omega = \omega\Delta t \rightarrow \infty$
    - varies in the range $0 \leq \rho_\infty \leq 1$
  - ➢ ρ_∞ = 1: No numerical energy dissipation
  - ➢ ρ_∞ = 0: Asymptotic annihilation
- Scalar integration parameters:
  - ➢ $\alpha_m = \frac{2}{\rho_\infty^3}$ … ; $\beta = \frac{1}{2}\left(\frac{1}{2}+\gamma\right)$

KR-α: One parameter (ρ_∞) family of algorithms

- Model-base…
  - ➢ $\boldsymbol{\alpha_1} = [\mathbf{M}_{IP} + \gamma\Delta t\mathbf{C}_{IP} + \beta\Delta t^2\mathbf{K}_{IP}]^{-1}\mathbf{M}_{IP}$; $\boldsymbol{\alpha_2} = \left(\frac{1}{2}+\gamma\right)\boldsymbol{\alpha_1}$
  - ➢ $\boldsymbol{\alpha_3} = [\mathbf{M}_{IP} + \gamma\Delta t\mathbf{C}_{IP} + \beta\Delta t^2\mathbf{K}_{IP}]^{-1}\left[\alpha_m\mathbf{M}_{IP} + \alpha_f\gamma\Delta t\mathbf{C}_{IP} + \alpha_f\beta\Delta t^2\mathbf{K}_{IP}\right]$
  - ➢ IP stands for integration parameters
  - ➢ $\mathbf{M}_{IP}$, $\mathbf{C}_{IP}$, and $\mathbf{K}_{IP}$ need to be formed based on the hybrid system

Kolay, C., and J.M. Ricles (2017) "Improved Explicit Integration Algorithms for Structural Dynamic Analysis with Unconditional Stability and Controller Numerical Dissipation," *Journal of Earthquake Engineering*, in press.

KR-α Method: Implementation for RTHS

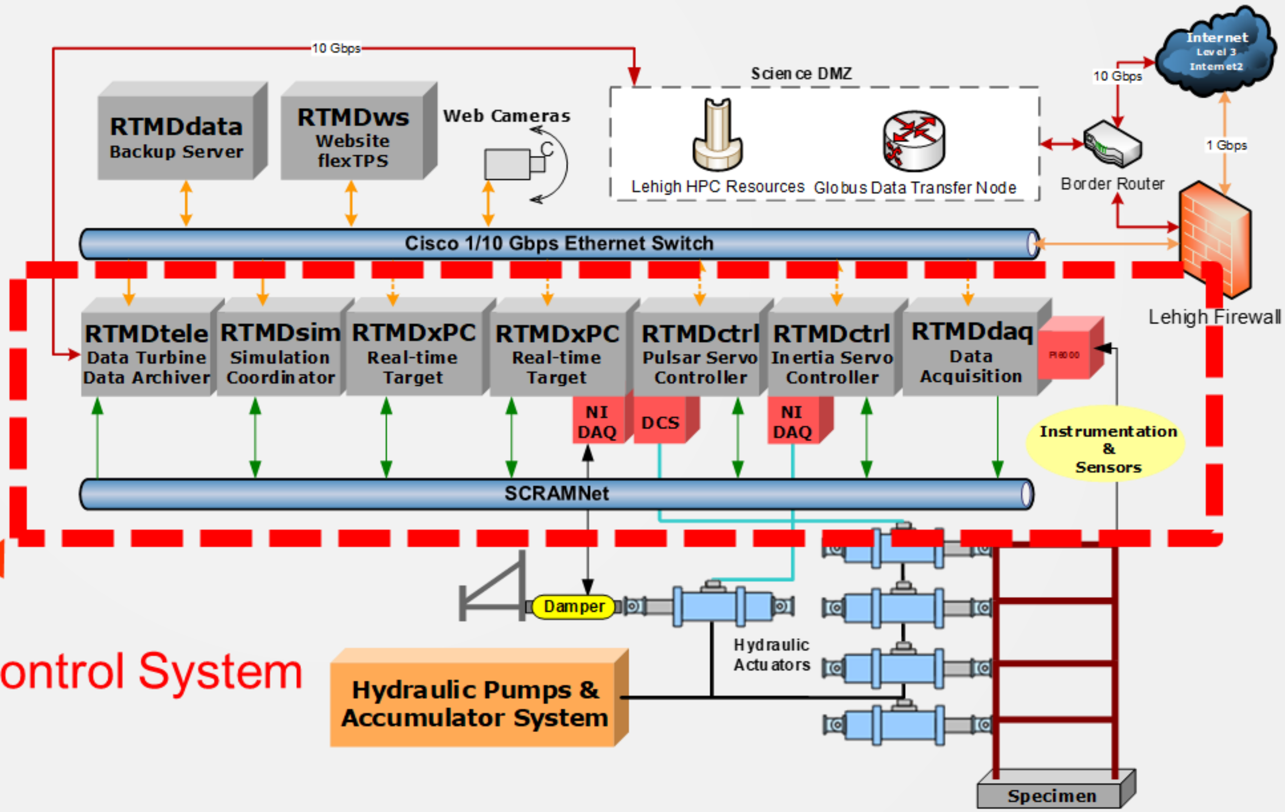


Kolay, C., Ricles, J., Marullo, T., Mahvashmohammadi, A., and Sause, R.. (2015). Implementation and application of the unconditionally stable explicit parametrically dissipative KR-α method for real-time hybrid simulation. *Earthquake Engineering & Structural Dynamics*. 44, 735-755, doi:10.1002/eqe.2484.

RTMD Real-time Integrated Control Architecture

NHERI Lehigh Solutions to RTHS Challenges

- Multiple real-time workstations with real-time communication (SCRAMNet)
- Synchronized control commands with simulation data, DAQ, and camera triggers to enable real time simulations and telepresence



Real-Time
Integrated Control System

LEHIGH UNIVERSITY. ATLSS RTMD REAL-TIME MULTI-DIRECTIONAL SIMULATION NATURAL HAZARDS ENGINEERING RESEARCH INFRASTRUCTURE NSF DESIGNSAFE-CI A NATURAL HAZARDS ENGINEERING COMMUNITY

RTHS: Implementation issues and challenges

Analytical substructure

- Fast and accurate state determination procedure

NHERI Lehigh Solutions

- Explicit force-based fiber elements
- HybridFEM
- Multi-grid real-time hybrid simulation

Fiber Element State Determination

FE Modeling of Analytical Substructure

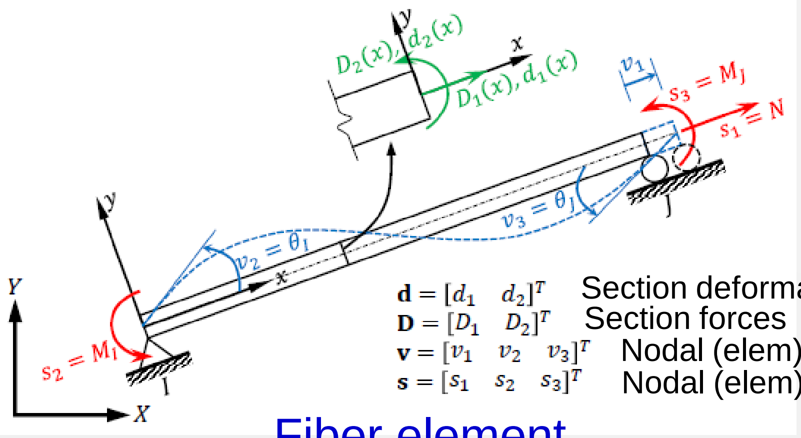
Displacement-based fiber elements

- ❑ Curvature varies linearly
  - ➢ Requires many elements per structural member to model nonlinear response
  - ➢ Increases number of DOFs
- ❑ State determination is straight forward

Force-based fiber elements

- ❑ Equilibrium is strictly enforced
  - ✓ Material nonlinearity can be modeled using a single element per structural member
  - ✓ Reduces number of DOFs
- ❑ Requires iterations at the element level

Jeopardizes explicit integration

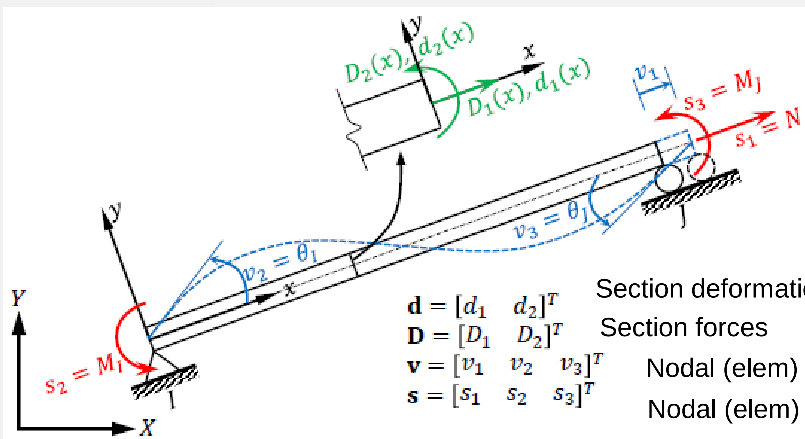


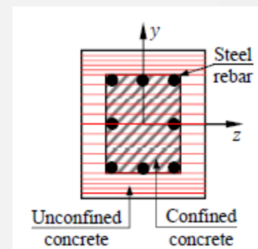
$\mathbf{d} = [d_1 \quad d_2]^T$ Section deformations
$\mathbf{D} = [D_1 \quad D_2]^T$ Section forces
$\mathbf{v} = [v_1 \quad v_2 \quad v_3]^T$ Nodal (elem) deformations
$\mathbf{s} = [s_1 \quad s_2 \quad s_3]^T$ Nodal (elem) forces

Fiber element

Explicit-formulated Force-Based Fiber Element

- Used with explicit integration algorithm
- Material nonlinearity
- Reduced DOFs in system modeling
- Fixed number of iterations during state determination with carry-over and correction of unbalanced section forces in next time step

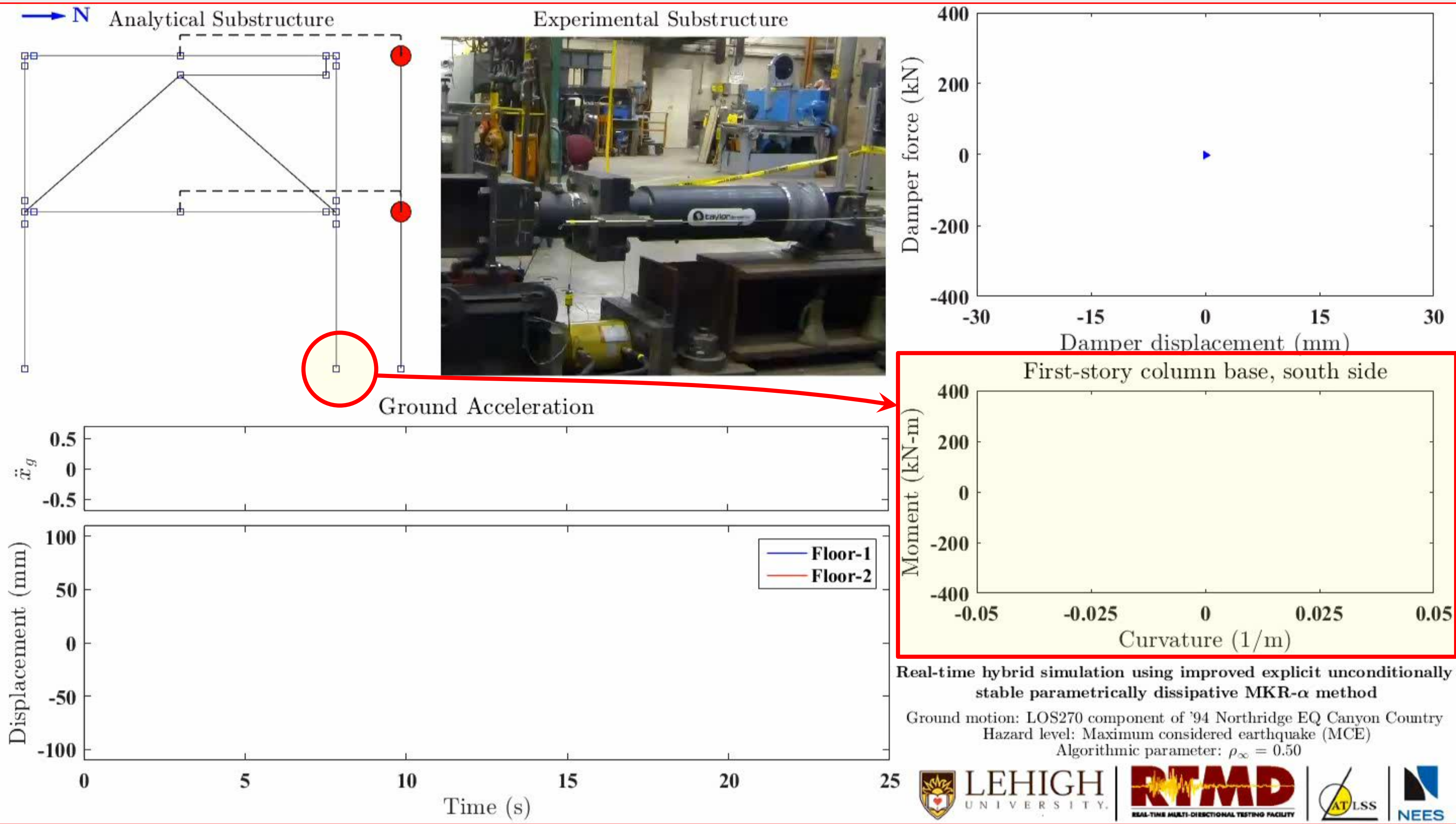




Fiber element

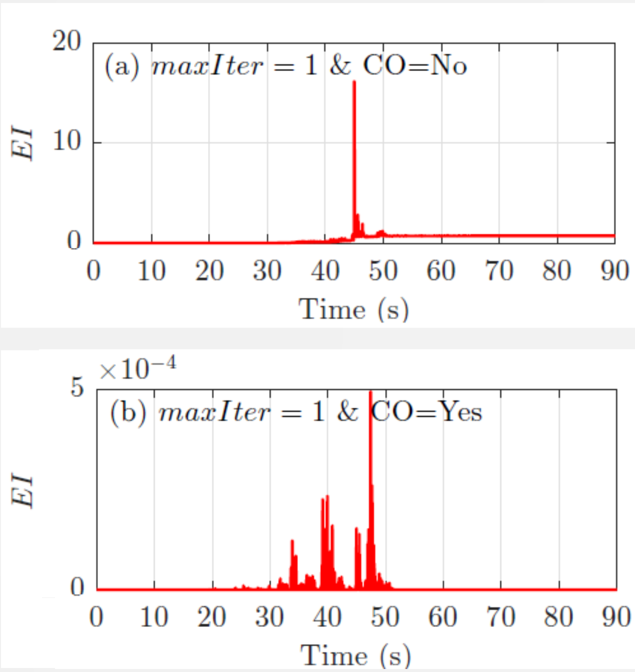
Kolay, C. and J.M. Ricles, (2017). Force-Based Frame Element Implementation for Real-Time Hybrid Simulation Using Explicit Direct Integration Algorithms. *Journal of Structural Engineering*, http://dx.doi.org/10.1080/13632469.2017.1326423.

RTHS of RC Structure: Fiber Element Real-time State-Determination

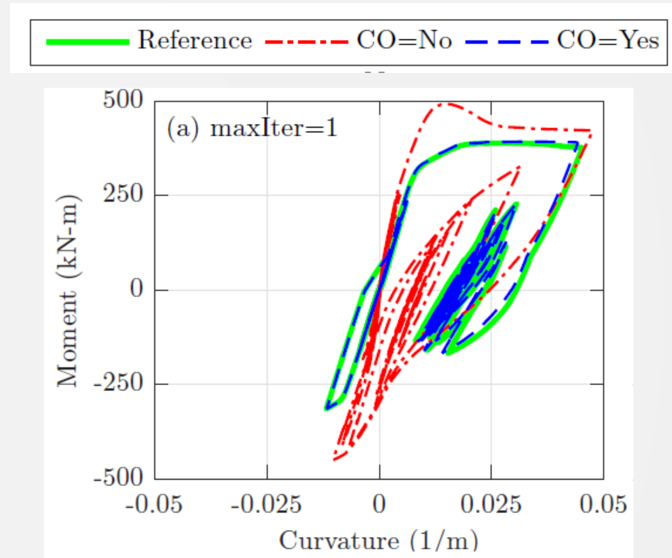


Explicit-formulated Force-Based Fiber Element

- Results



Energy Increment (EI) Error



Moment Curvature Response – 1st story RC column
(CO: Carry over unbalanced section forces)

Note: Reference = Newmark Constant Acceleration Method

Kolay, C., Ricles, J., (2017). Force-Based Frame Element Implementation for Real-Time Hybrid Simulation Using Explicit Direct Integration Algorithms. *Journal of Structural Engineering*, in press.

Lehigh HybridFEM

NHERI Lehigh Solutions to RTHS Challenges

- MATLAB and SIMULINK based computational modeling and simulation coordinator software for dynamic time history analysis and real-time hybrid simulation of inelastic-framed structures
- Run Modes
  - ➢ MATLAB script for numerical simulation
  - ➢ SIMULINK modeling for Real-Time Hybrid simulation with experimental elements via xPCs, and hydraulics-off for training and validation of user algorithms.
- User's Manual for training

Karavasilis, T. L., Seo, C.-Y., & Ricles, J. M. (2012). *HybridFEM: A program for dynamic time history analysis and real-time hybrid simulation* (ATLSS Report). *ATLSS Report* (Vol. 08–09). Bethlehem, PA.

Lehigh HybridFEM

Configuration Options:

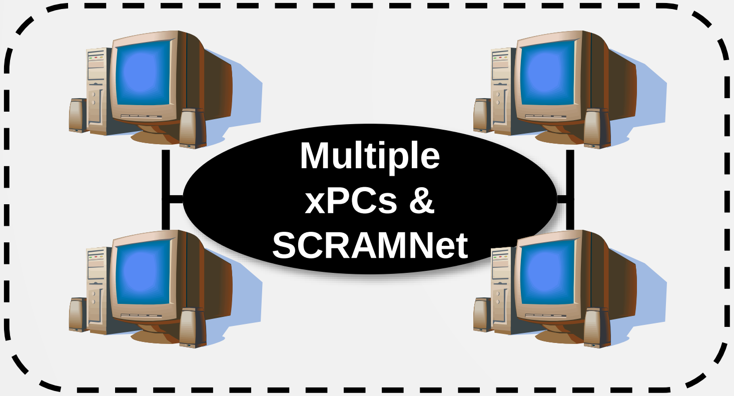
- Coordinate system of nodes
- Boundary, constraint and restraint conditions
- Elements
  - Elastic beam-column
  - Elastic spring
  - Inelastic beam-column stress resultant element
  - Non-linear spring
  - Non-linear truss element
  - Displacement-based NL beam-column fiber element
  - Force-based NL beam-column fiber element
  - Zero-length
  - 2D NL planar panel zone
  - Elastic beam-column element with geometric stiffness
- Geometric nonlinearities
- Steel wide flange sections (link to AISC shapes Database)
- Reinforced concrete sections
- Structural mass & inherent damping properties
- Adaptable integration methods

- Materials
  - Elastic
  - Bilinear elasto-plastic
  - Hysteretic
  - Bouc-Wen
  - Trilinear
  - Stiffness degrading
  - Concrete
  - Steel

Multi-grid real-time hybrid simulation

NHERI Lehigh Solutions to RTHS Challenges

- Parallel computing method used with multiple xPCs and SCRAMNet to improve the computational speed for complex large structures
- Incorporated into RTMD Real-time Integrated Control Architecture



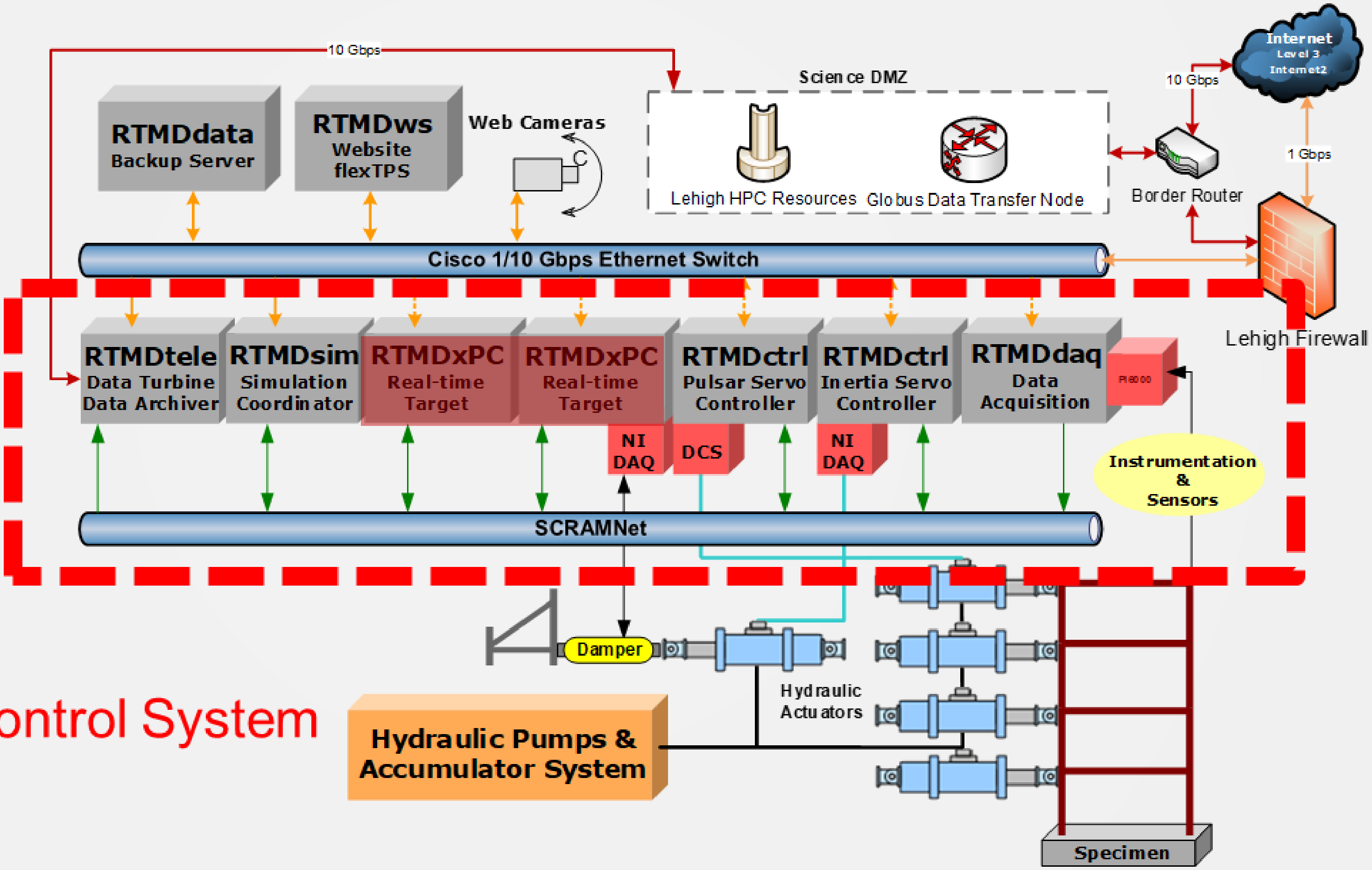
Solve equations of motion with multiple xPCs and communication via SCRAMNet

+

Experimental substructure

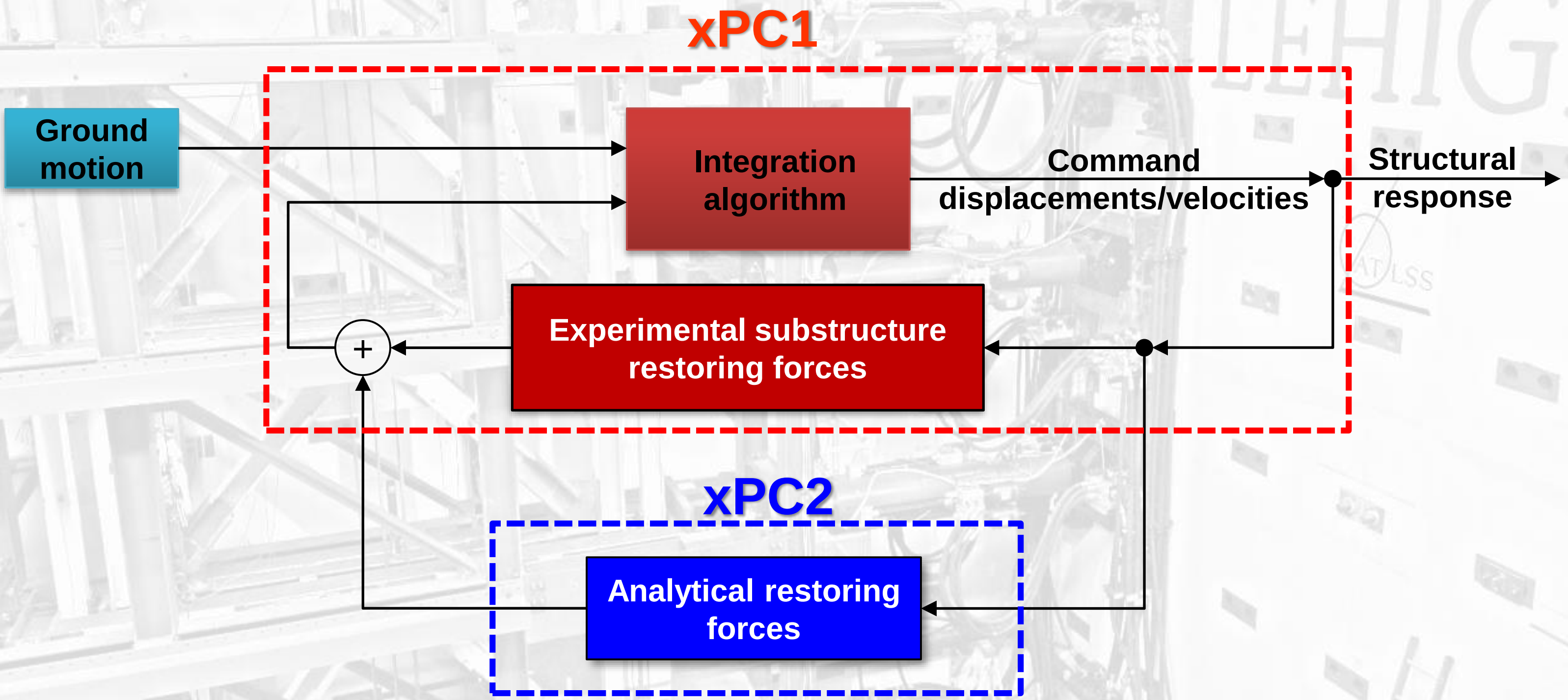
Chae, Y., Tong, S., Marullo, T., and Ricles, J.M. (2012). "Real-time hybrid simulation studies of complex large-scale systems using multi-grid processing." *20th Analysis and Computation Specialty Conference*, Chicago, IL.

RTMD Real-time Integrated Control Architecture

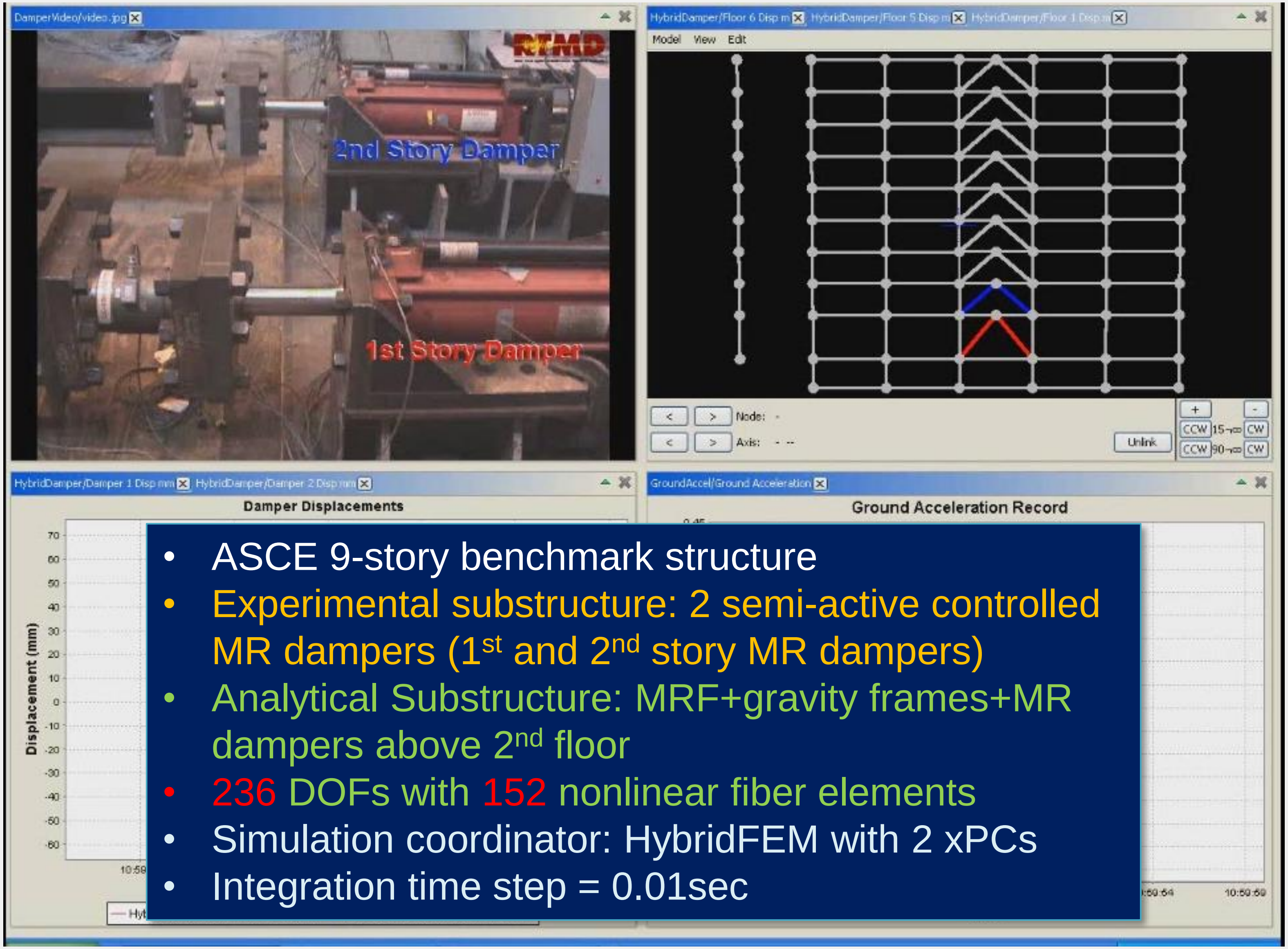


Real-Time
Integrated Control System

Multi-Grid Real-Time Hybrid Simulation



Multi-Grid RTHS: 9-story Steel Frame with Semi-active Controlled MR Dampers



- ASCE 9-story benchmark structure
- Experimental substructure: 2 semi-active controlled MR dampers (1st and 2nd story MR dampers)
- Analytical Substructure: MRF+gravity frames+MR dampers above 2nd floor
- 236 DOFs with 152 nonlinear fiber elements
- Simulation coordinator: HybridFEM with 2 xPCs
- Integration time step = 0.01sec

RTHS: Implementation issues and challenges

Experimental substructure

- Large capacity hydraulic system and dynamic actuators required
- Actuator kinematic compensation
- Robust control of dynamic actuators for large-scale structures

NHERI Lehigh Solutions

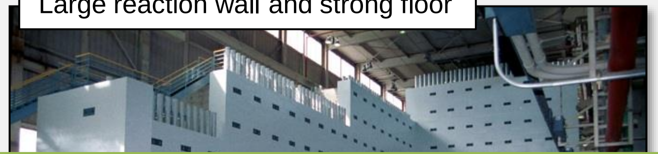
- Large hydraulic power supply system
- 5 large capacity dynamic actuators
- Development of actuator kinematic compensation
- Servo hydraulic actuator control: Adaptive Time Series Compensator (ATS)

Large Capacity Hydraulic System and Dynamic Actuators

NHERI Lehigh Solutions to RTHS Challenges

- Lehigh has unique equipment with large hydraulic power, facilitating large-scale real-time hybrid simulation

Large reaction wall and strong floor

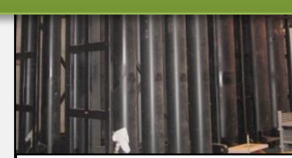


Large-force capacity dynamic actuators

Maximum load capacity
 2 actuators: 517 kips (2,300kN)
 3 actuators: 382 kips (1,700kN)

Stroke
 +/- 20 in (+/- 500mm)

Maximum velocity
 45 in/s (1,140mm/sec) for 382 kip actuators
 33 in/s (840mm/sec) for 517 kip actuators



Accumulator System

DAQ System

Large Capacity Hydraulic System and Dynamic Actuators

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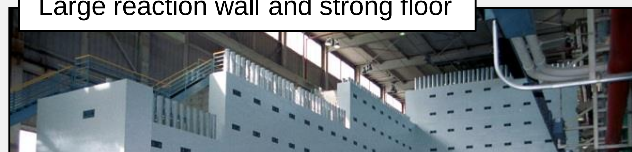
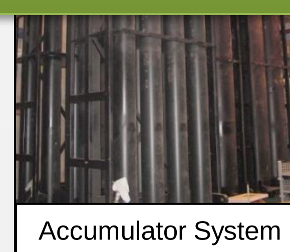
Large-force capacity dynamic actuators



Large reaction wall and strong floor

- Enables a large-scale RTHS of a structure under strong ground motions (i.e., Kobe earthquake, Japan)
- Collapse simulation of a building structure was conducted under extreme earthquake ground motions (beyond MCE level)



Accumulator System

DAQ System

Actuator Kinematic Compensation

- Develop kinematic compensation scheme and implementation for RTHS (Mercan et al. 2009)
  - Kinematic correction of command displacements for multi-directional actuator motions
  - Robust, avoiding accumulation of error over multiple time steps; suited for RTHS
  - Exact solution for planar motions

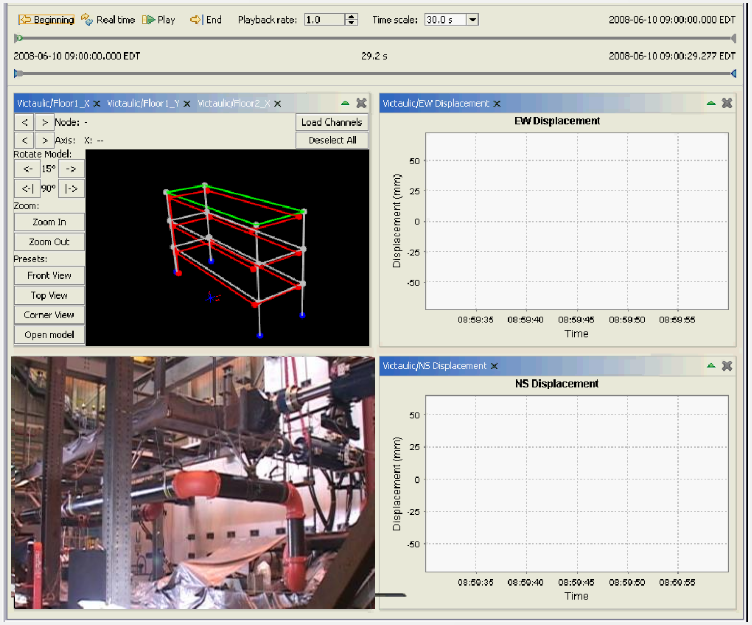
$$(M_i SNxL_{new}, M_i SNyL_{new}) = (-LMa_{inew}\sin(\Theta_2+\phi_i), LMa_{inew}\cos(\Theta_2+\phi_i))$$

$$\Theta_2 = \arcsin\left[\frac{LMb_{inew}}{yF_i/\cos\phi_i}\sin\Theta_3\right]$$

$$\Theta_3 = \arccos\left[\frac{LMa_{inew}^2 + LMb_{inew}^2 - (yF_i/\cos\phi_i)^2}{2LMa_{inew}LMb_{inew}}\right]$$

$$(SPN^m x_{new}, SPN^m y_{new}) = (M_1SN^m x_{new} - \left|\overrightarrow{VM}_1\right|\cos(\Theta M_{1,0}+d^m SPN\Theta), M_1SN^m y_{new} - \left|\overrightarrow{VM}_1\right|\sin(\Theta M_{1,0}+d^m SPN\Theta))$$

Mercan, O, Ricles, J.M., Sause, R, and M. Marullo, "Kinematic Transformations in Planar Multi-directional Pseudo-Dynamic Testing," *Earthquake Engineering and Structural Dynamics*, Vol. 38(9), pp. 1093-1119, 2009.



Multi-directional Real-time Hybrid Simulation

Servo Hydraulic Actuator Control

Sources of Nonlinearity in Real-Time Hybrid Simulation

- Nonlinear servo-valve dynamics
- Nonlinear actuator fluid dynamics
- Test specimen material and geometric nonlinearities
- Slop, misalignment, deformations in test setup

Can lead to variable amplitude error and time delay in servo-hydraulic system that does not enable the target displacement of the experimental substructure to be achieved

Effect of time delay on real-time hybrid simulation

- Inaccurate structural response
- Delayed restoring force adds energy into the system (negative damping)
- Can cause the instability of simulation

➔ *important to negate the time delay effect in real-time hybrid simulation*

Servo Hydraulic Actuator Control

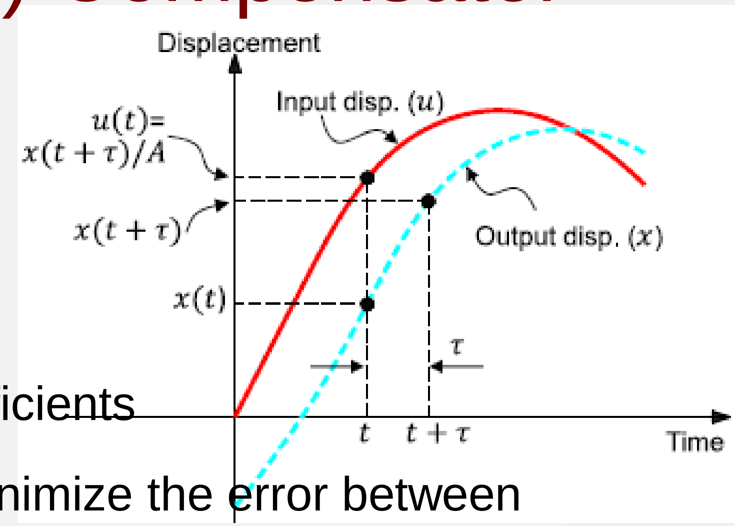
- Actuator delay compensation
  - Inverse compensation (Chen 2007)
  - Adaptive inverse compensation (AIC, Chen and Ricles 2010)
  - Adaptive time series (ATS) compensator (Chae et al. 2013)

- Chae, Y., Kazemibidokhti, K., and Ricles, J.M. (2013). “Adaptive time series compensator for delay compensation of servo-hydraulic actuator systems for real-time hybrid simulation”, *Earthquake Engineering and Structural Dynamics* (accepted for publication).
- Chen C. Development and numerical simulation of hybrid effective force testing method. *Ph.D. Dissertation, Department of Civil and Environmental Engineering, Lehigh University, Bethlehem, PA* 2007.
- Chen, C. and Ricles, J.M. Tracking error-based servohydraulic actuator adaptive compensation for real-time hybrid simulation. *ASCE Journal of Structural Engineering*, 2010; **136**(4):432-440.

Adaptive Time Series (ATS) Compensator

2nd order ATS compensator

$$u_k^c = a_{0k} x_k^t + a_{jk} \dot{x}_k^t + a_{2k} \ddot{x}_k^t$$



u_k^c: compensated input displacement into actuator

x_k^t: target **specimen** displacement a_{jk}: adaptive coefficients

Adaptive coefficients are optimally updated to minimize the error between the specimen target and measured displacements using the **least squares** method

$$\mathbf{A} = \left(\mathbf{X}_m^T \mathbf{X}_m\right)^{-1} \mathbf{X}_m^T \mathbf{U}_c$$

$$\mathbf{A} = \left[a_{0k}\ a_{1k} \cdots a_{nk}\right]^T \qquad \mathbf{X}_m = \left[\mathbf{x}^m\ \dot{\mathbf{x}}^m \cdots \frac{d^n}{dt^n}\left(\mathbf{x}^m\right)\right]^T$$

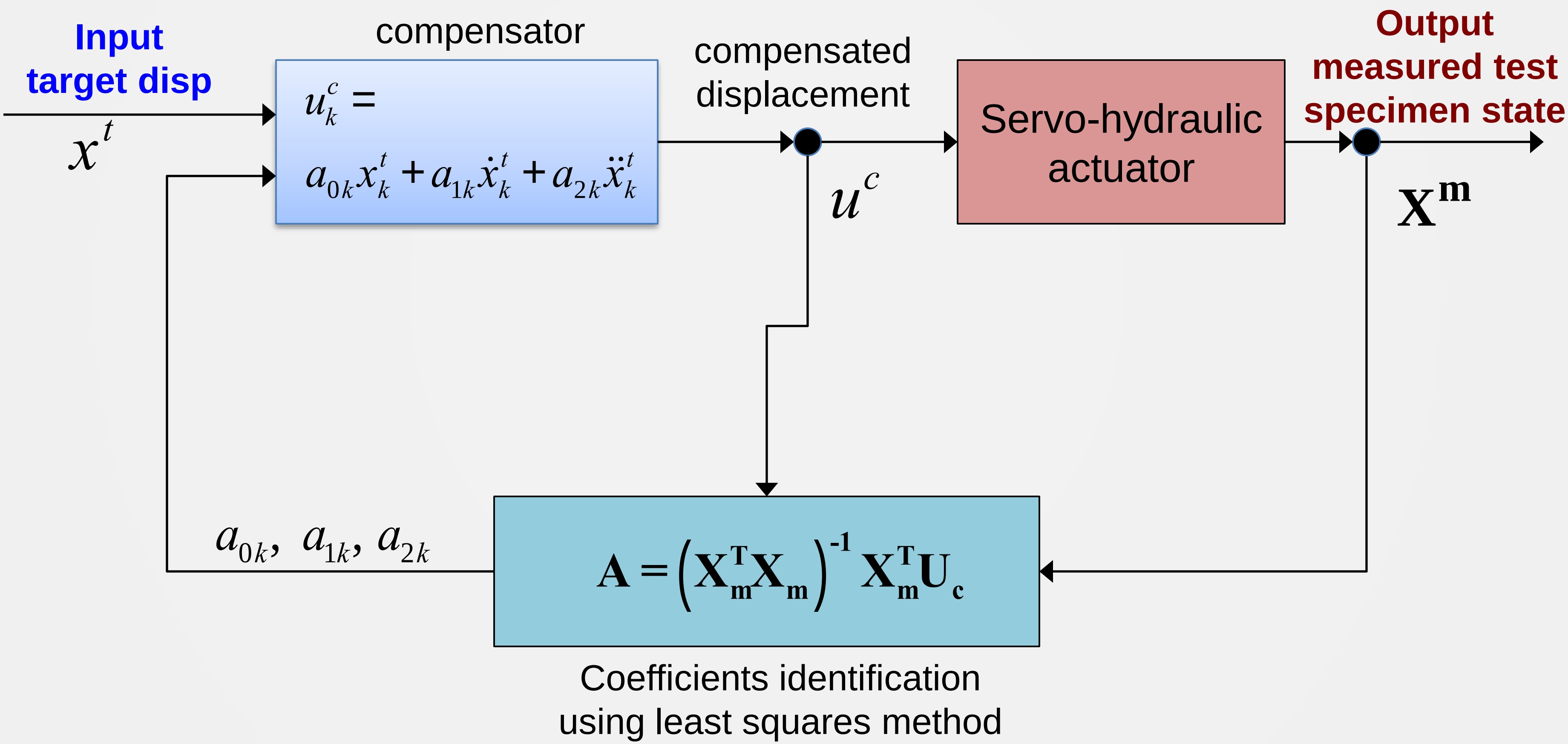
$$\mathbf{x}^m = \left[x_{k-1}^m\ x_{k-2}^m \cdots x_{k-q}^m\right]^T$$ (Output (measured) specimen displacement history)

$$\mathbf{U}_c = \left[u_{k-1}^c\ u_{k-2}^c \cdots u_{k-q}^m\right]^T$$ (Input actuator displacement history)

Chae, Y., Kazemibidokhti, K., and Ricles, J.M. (2013). "Adaptive time series compensator for delay compensation of servo-hydraulic actuator systems for real-time hybrid simulation", *Earthquake Engineering and Structural Dynamics,* DOI: 10.1002/ eqe.2294.

Adaptive Time Series (ATS) Compensator

Block Diagram



Adaptive Time Series (ATS) Compensator

Unique features of ATS compensator

- No user-defined adaptive gains ➔ applicable for large-scale structures susceptible to damage (i.e., concrete structures)
- Negates both variable time delay and variable amplitude error response
- Time delay and amplitude response factor can be easily estimated from the identified values of the coefficients
- Use specimen feedback

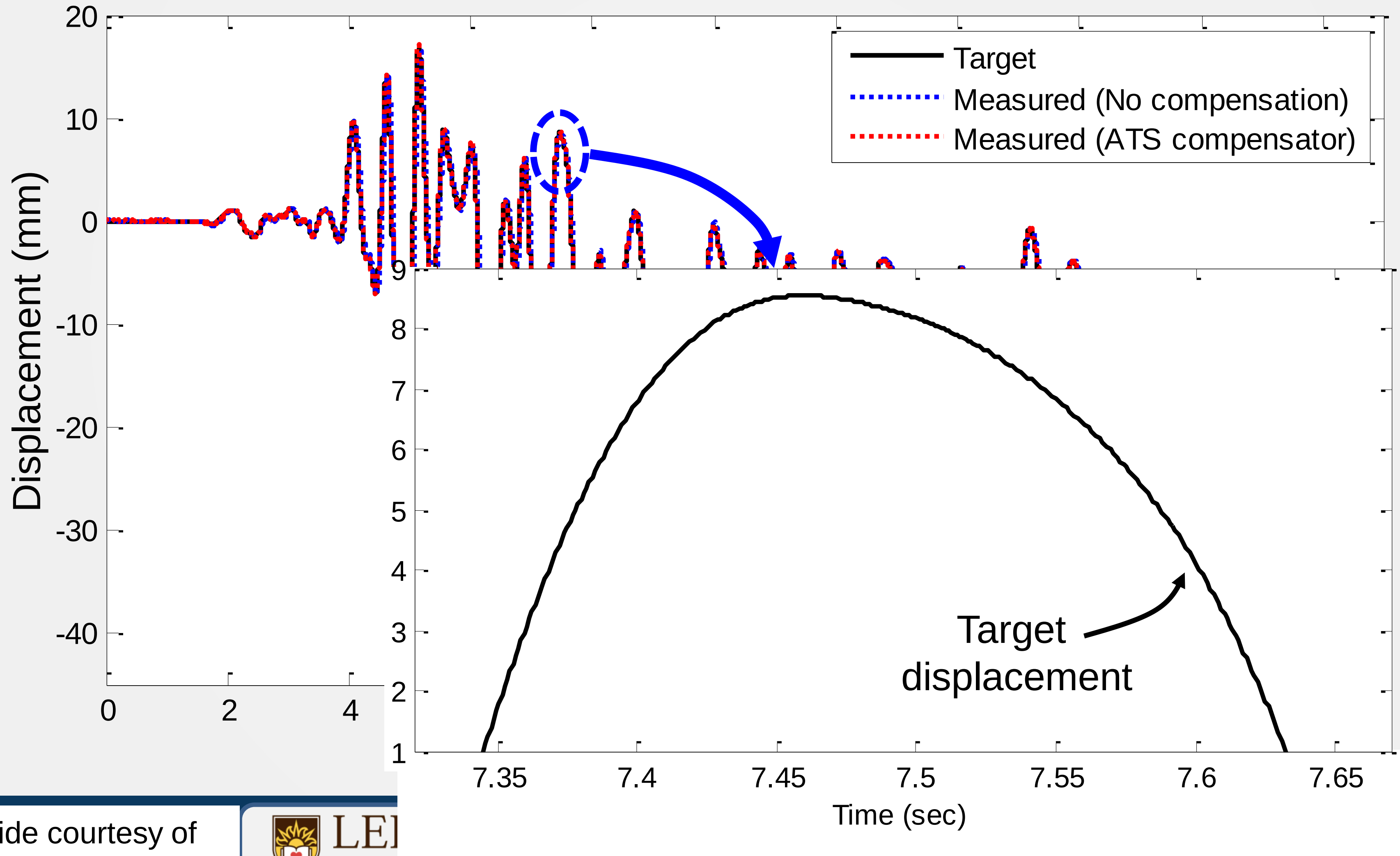
Amplitude error: $$A = \frac{1}{a_{0k}}$$

Time delay: $$t = \frac{a_{1k}}{a_{0k}}$$

Adaptive Time Series (ATS) Compensator

- Performance of ATS compensator -

Predefined EQ displacement test (maximum amplitude=40mm)

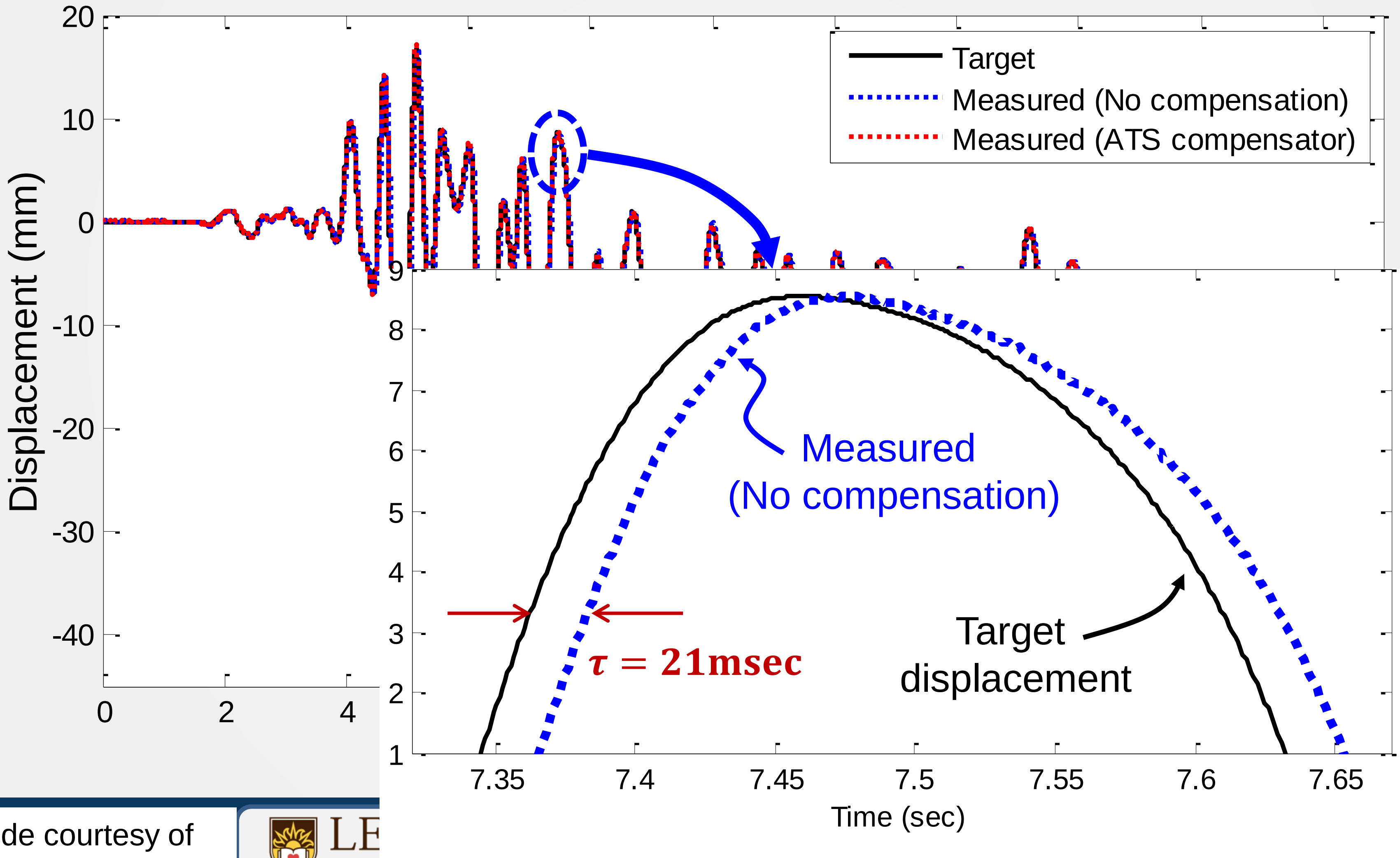


Slide courtesy of Yunbyeong Chae

Adaptive Time Series (ATS) Compensator

- Performance of ATS compensator -

Predefined EQ displacement test (maximum amplitude=40mm)

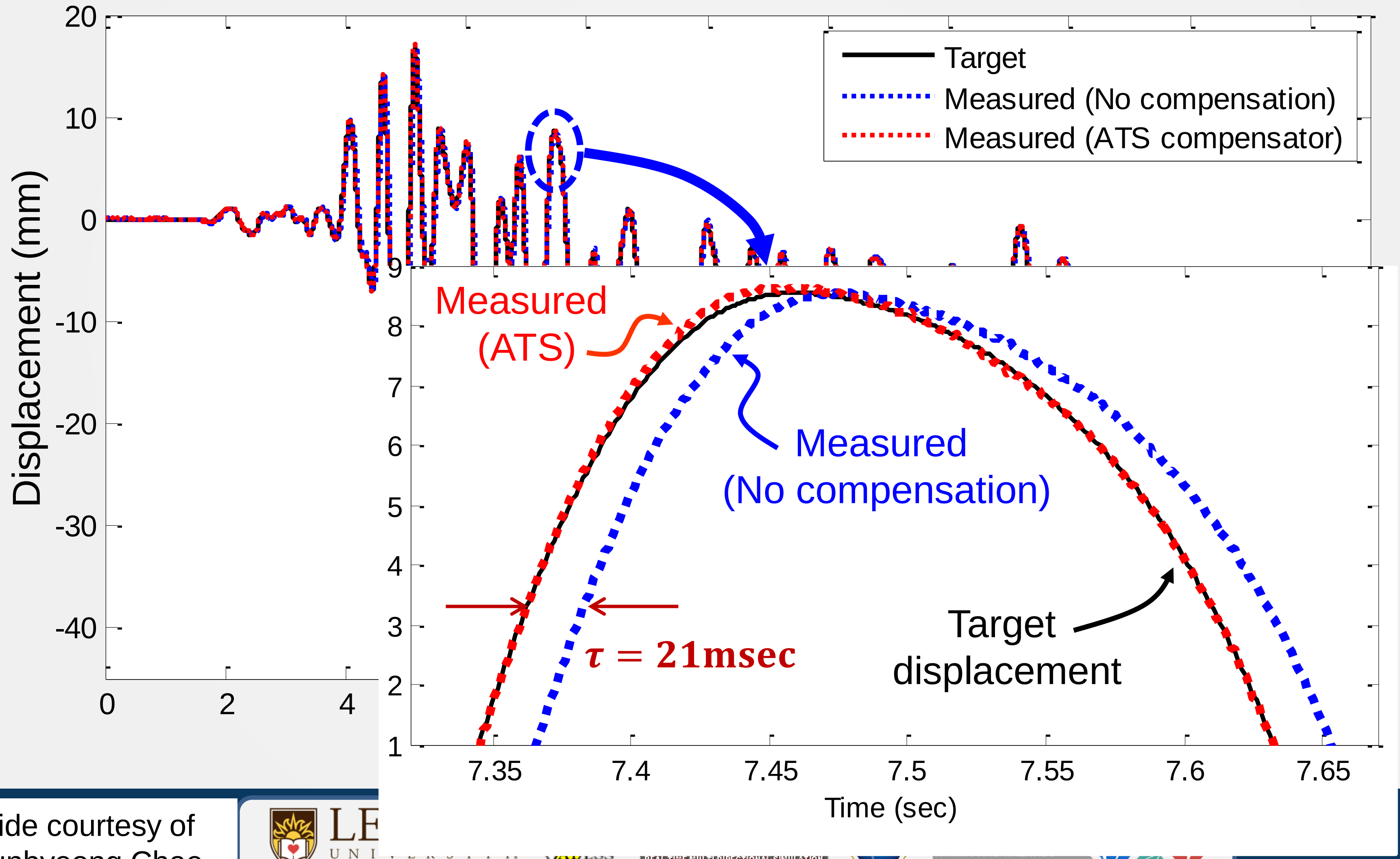


Slide courtesy of
Yunbyeong Chae

Adaptive Time Series (ATS) Compensator

- Performance of ATS compensator -

Predefined EQ displacement test (maximum amplitude=40mm)

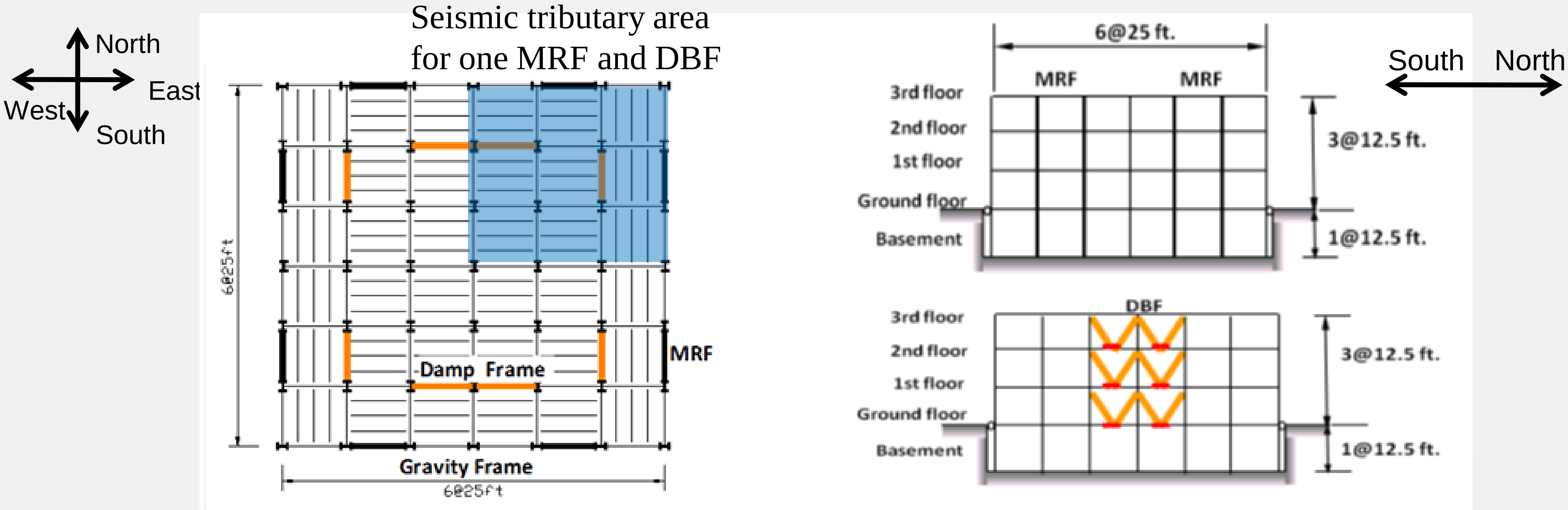


Slide courtesy of
Yunbyeong Chae

Applications of RTHS to Resilient Systems

Steel Frame Building with Nonlinear Viscous Dampers Prototype Building

- 3-story, 6-bay by 6-bay office building located in Southern California
- Seismic design category D
- Moment resisting frame (MRF); damped braced frame (DBF), gravity system



Plane View of 3-Story Prototype Building **Elevations of 3-Story Prototype Building**

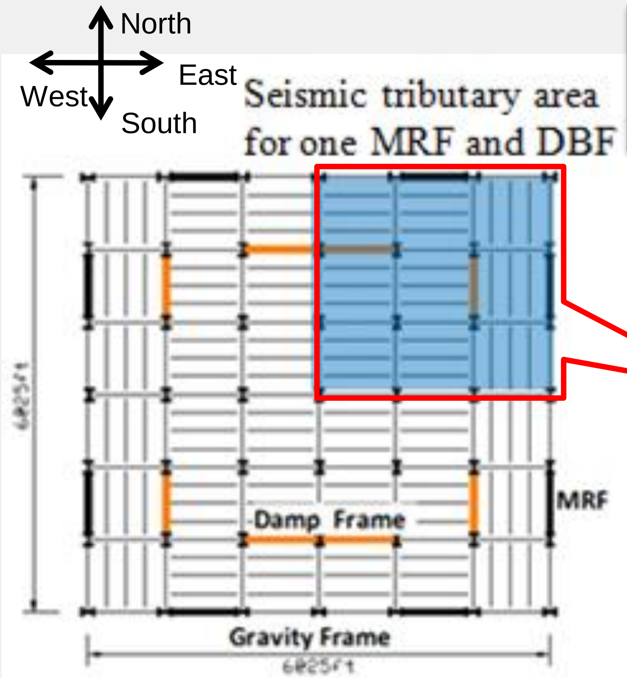
Dong, B. "Large-scale Experimental, Numerical, and Design Studies of Steel MRF Structures with Nonlinear Viscous Dampers under Seismic Loading", *PhD Dissertation*, Department of Civil and Environmental Engineering, Lehigh University, Bethlehem, PA 2015.

Prototype and Test Structure

- MRFs designed to satisfy ASCE7 code strength requirement
- Story drift controlled by nonlinear elastomeric dampers installed in DBFs
- DBFs designed to remain elastic under design basis earthquake (DBE) ground motion
- Test structures derived by scaling down the prototype by a factor of 0.6

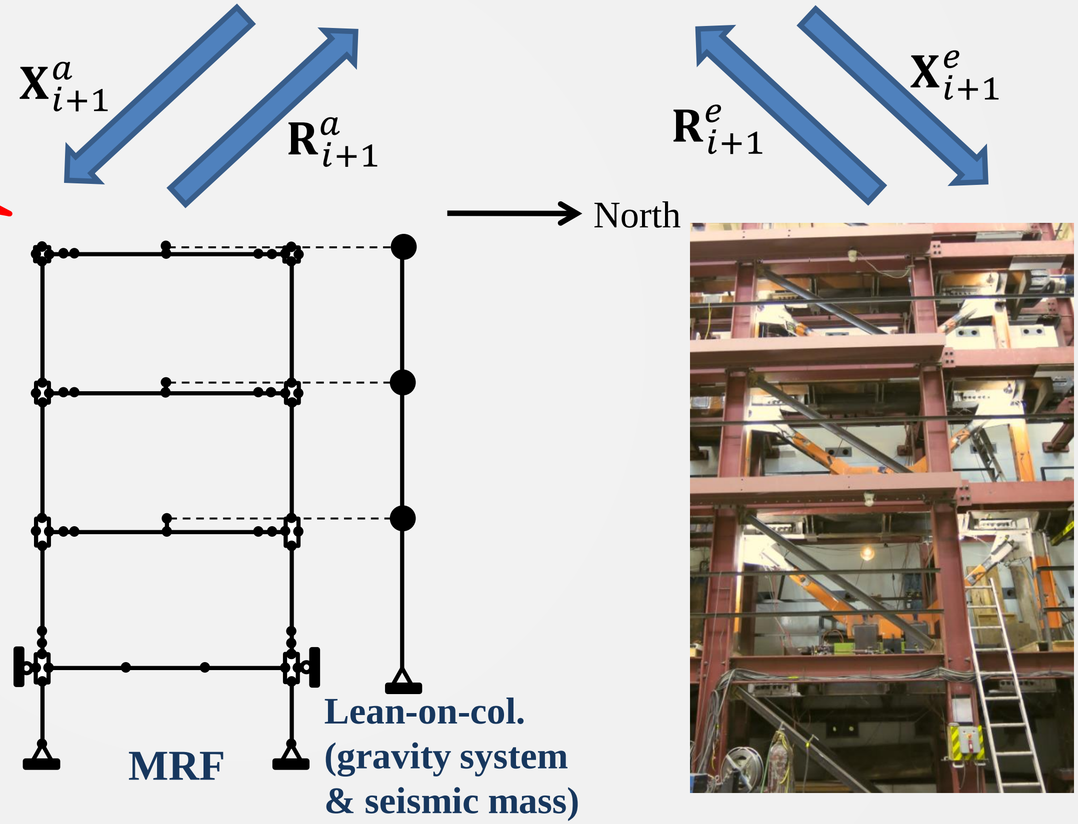
Mahvashmohammadi, A. "Design and Assessment of Supplemental Elastomeric Dampers for Improved Seismic Performance of New Buildings", *PhD Dissertation*, Department of Civil and Environmental Engineering, Lehigh University, Bethlehem, PA 2015.

Substructures for RTHS



Time discretized weighted equation of motion (MKR−α Method):

$$\mathbf{M}\hat{\ddot{\mathbf{X}}}_{i+1} + \mathbf{C}\dot{\mathbf{X}}_{i+1-\alpha_f} + (\mathbf{R}^a_{i+1-\alpha_f} + \mathbf{R}^e_{i+1-\alpha_f}) = \mathbf{F}_{i+1-\alpha_f}$$

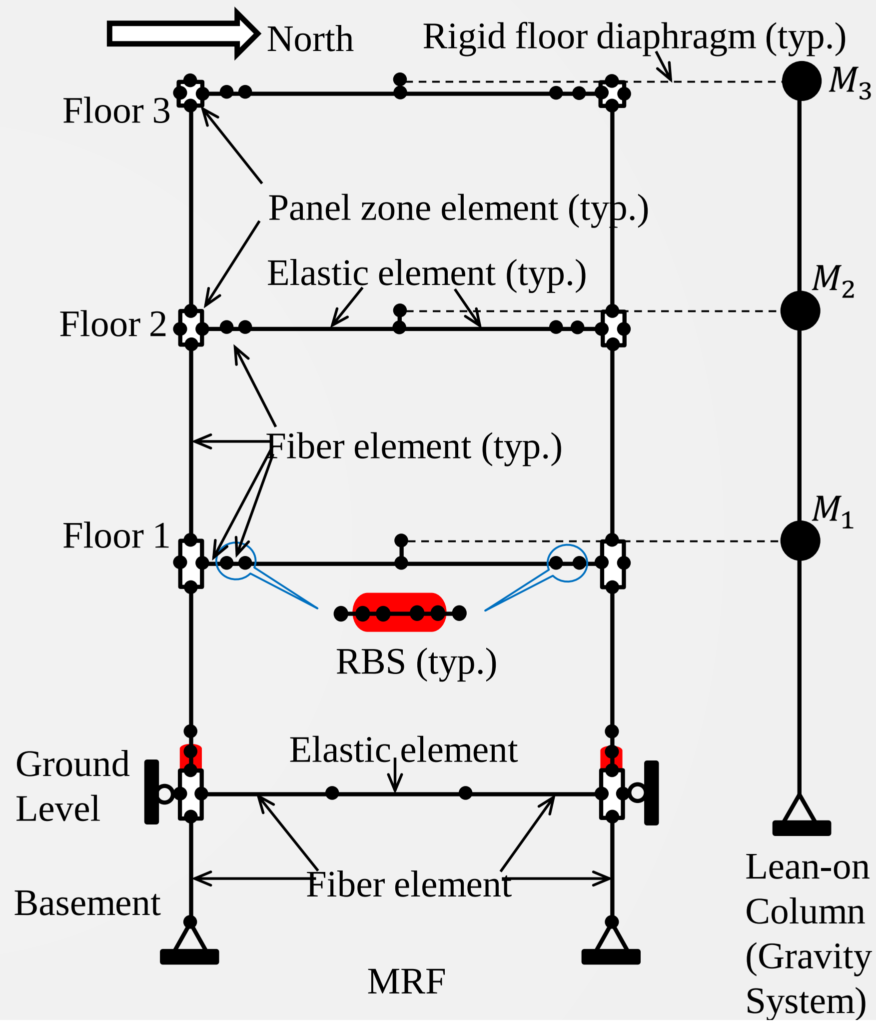


Analytical Substructure

Experimental Substructure (DBF)

Analytical Substructure

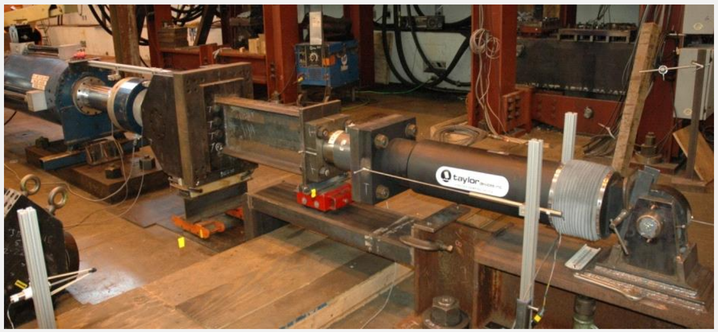
- FE model developed in HybridFEM
- Columns and beams
  - displacement-based nonlinear beam-column fiber elements and elastic beam-column elements
- MRF panel zone
  - nonlinear panel-zone elements
- Nonproportional damping (NPD) model
- Gravity system
  - lean-on-column using elastic elements with second order $P-\Delta$ effects
- 247 DOFs and 74 elements



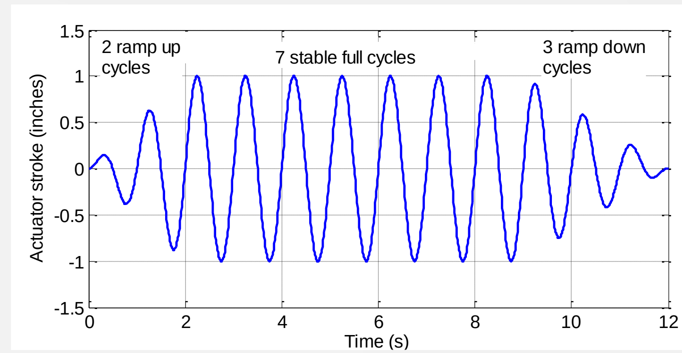
Karavasilis, T. L., Seo, C.-Y., & Ricles, J. M. (2012). *HybridFEM: A program for dynamic time history analysis and real-time hybrid simulation* (ATLSS Report). *ATLSS Report* (Vol. 08–09). Bethlehem, PA.

Nonlinear Viscous Dampers

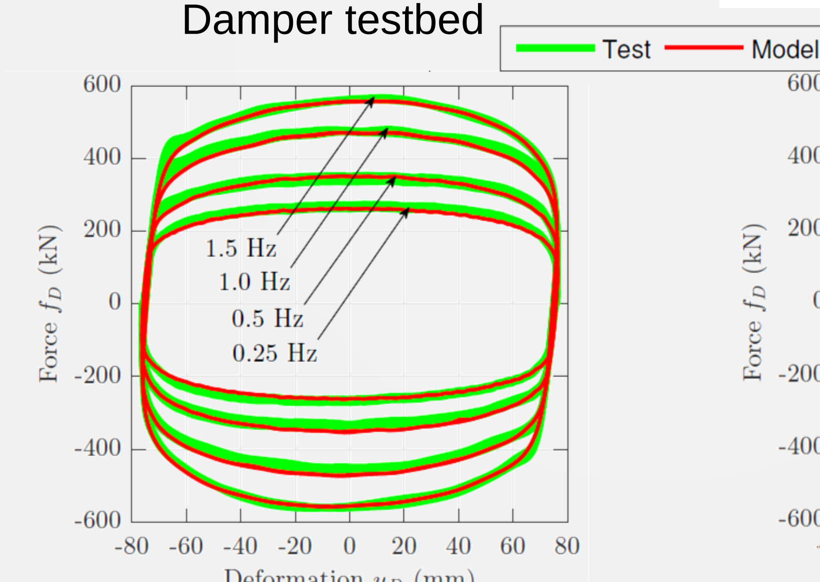
Characterization testing



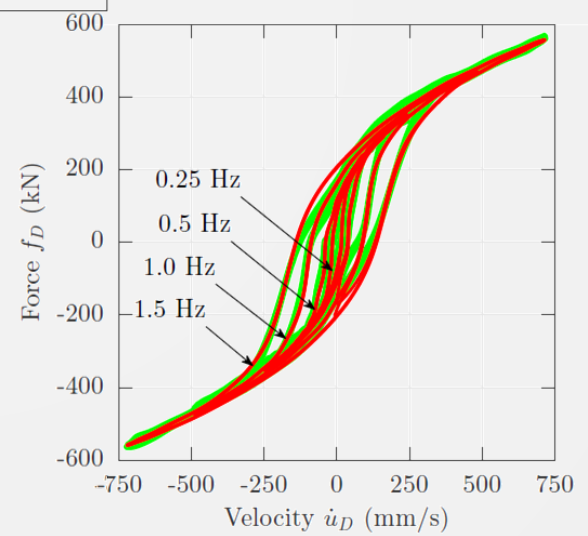
Damper testbed



Loading Protocol



Damper force - deformation



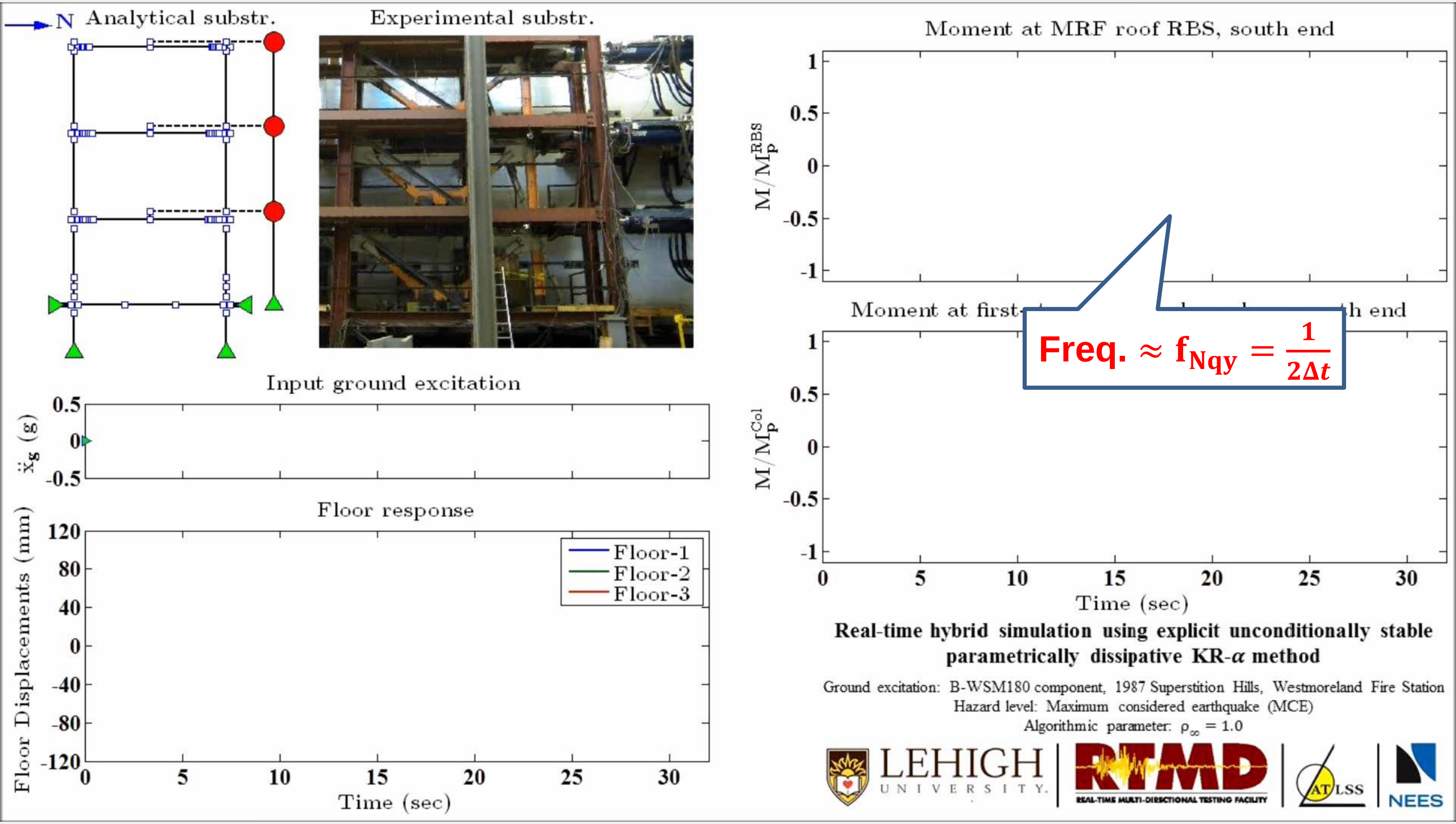
Damper force - velocity

RTHS: Ground motion and time step

- Ground motion
  - B-WSM180 component of the 1987 Superstition Hills earthquake recorded at the Westmoreland Fire Station
  - Chosen from a suit of 20 ground motion records which produce a median spectral acceleration that matches the design spectrum in the period range of 0.2 – 2.0 sec.
  - Scaled to two hazard levels
    - Design basis earthquake (DBE)*: Scale factor = 1.51
    - Maximum considered earthquake (MCE)*: Scale factor = 2.26
- Time step
  - $\Delta = \frac{4}{1024}$ sec, the smallest time step within which the numerical computation can be finished in real-time

*Note: DBE has 475 year return period (10% probability of exceedance in 50 years)
MCE has 2475 year return period (2% probability of exceedance in 50 years)

MCE level RTHS using $\rho_\infty = 1.0$

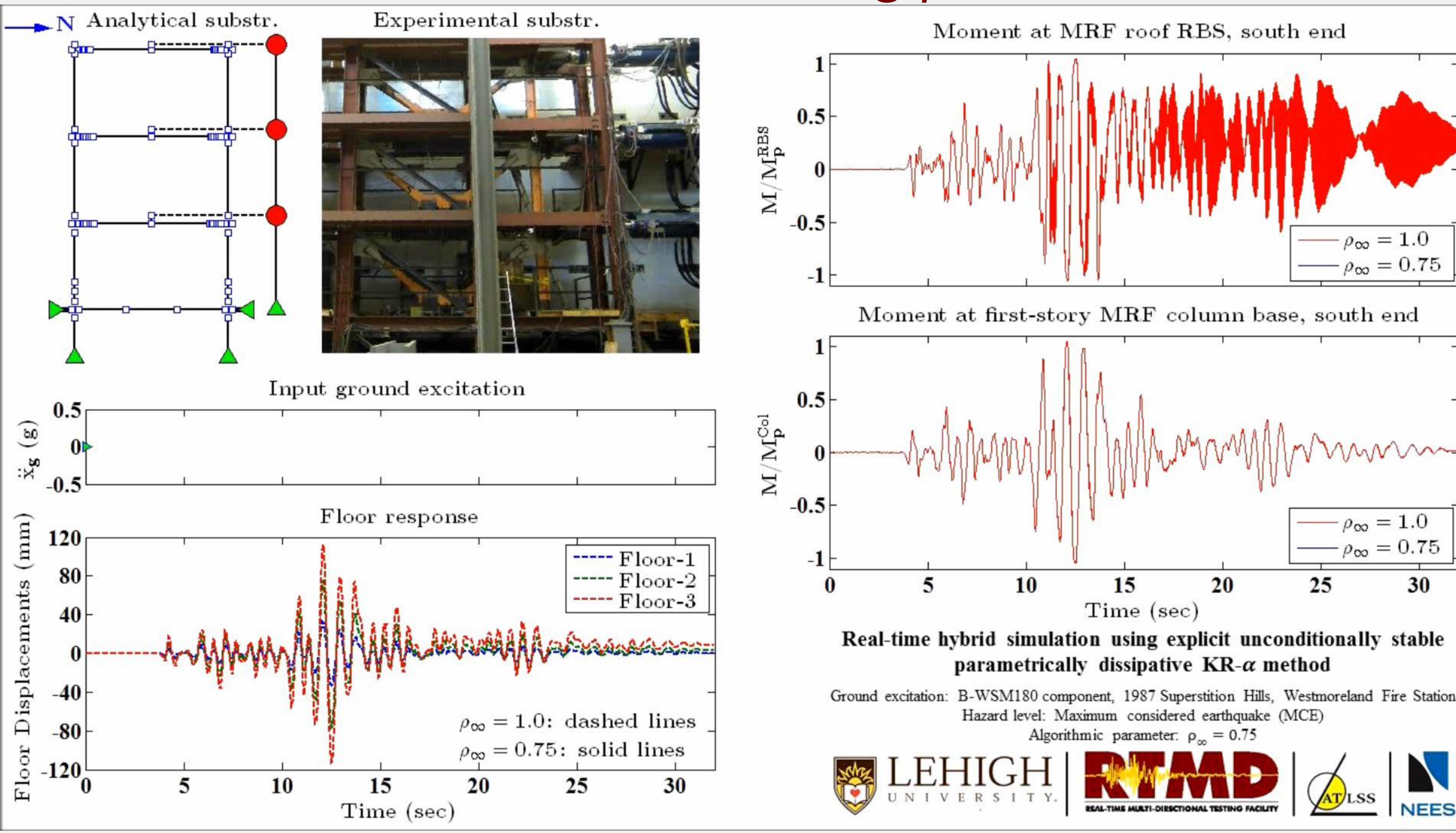


Kolay, C., Ricles, J., Marullo, T., Mahvashmohammadi, A., and Sause, R.. (2015). Implementation and application of the unconditionally stable explicit parametrically dissipative KR-α method for real-time hybrid simulation. *Earthquake Engineering & Structural Dynamics*. 44, 735-755, doi:10.1002/eqe.2484.

High frequency oscillations in member forces

- Under nonlinear structural behavior, pulses are introduced in the acceleration at the Nyquist frequency $\left(=\frac{1}{2\Delta t}\right)$ when the state of the structure changes within the time step
- These pulses excite spurious higher modes present in the system which primarily contribute to the member forces
- The problem becomes worst by the noise introduced through the measured restoring forces and the actuator delay compensation which can amplify high frequency noise.
- How can we remove them?
  - Reduce the time step: Not always possible due to the computation time required for each time step
  - Introduce controllable numerical damping

MCE level RTHS using $\rho_\infty = 0.75$



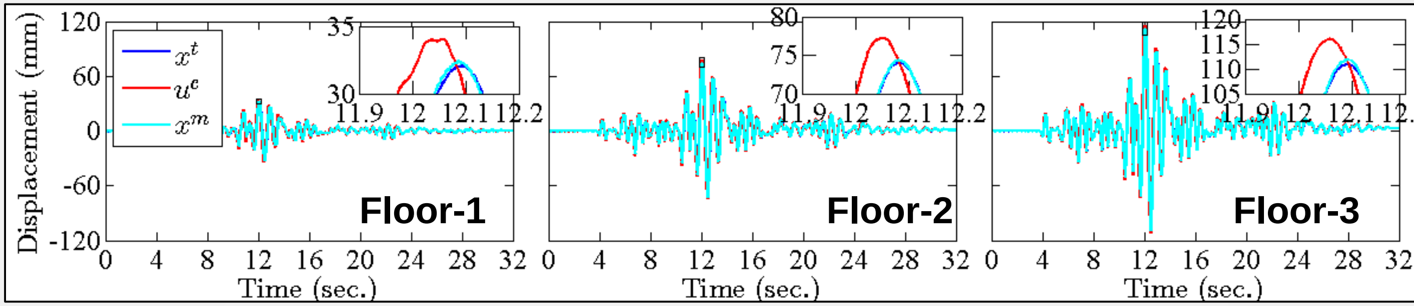
Kolay, C., Ricles, J., Marullo, T., Mahvashmohammadi, A., and Sause, R.. (2015). Implementation and application of the unconditionally stable explicit parametrically dissipative KR-α method for real-time hybrid simulation. *Earthquake Engineering & Structural Dynamics*. 44, 735-755, doi:10.1002/eqe.2484.

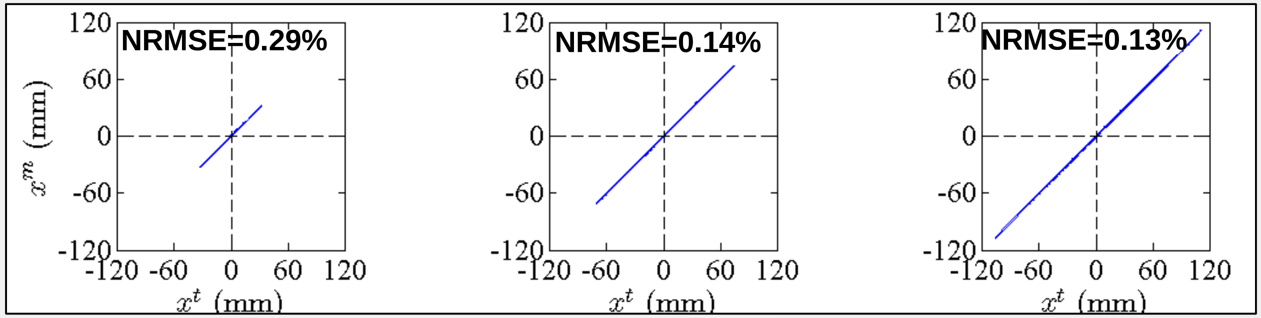
Actuator control: Typical MCE level test & $\rho_\infty = 0.75$

x^t : targeted specimen displacement

u^c : input command to actuator

x^m : measured specimen displacement





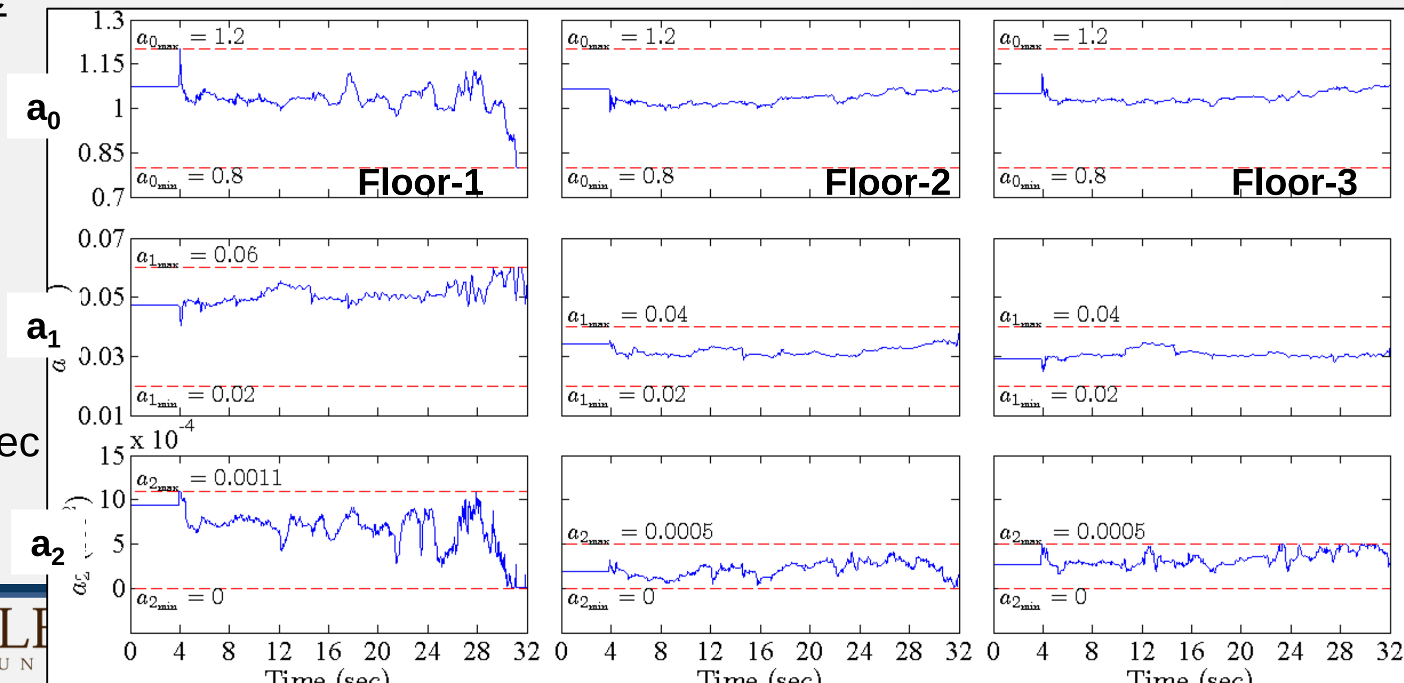
Time History of Adaptive Coefficients

Amplitude Correction

$$A_k^{(j)} \approx \frac{1}{a_{0k}^{(j)}} = 0.83 \sim 1.25$$

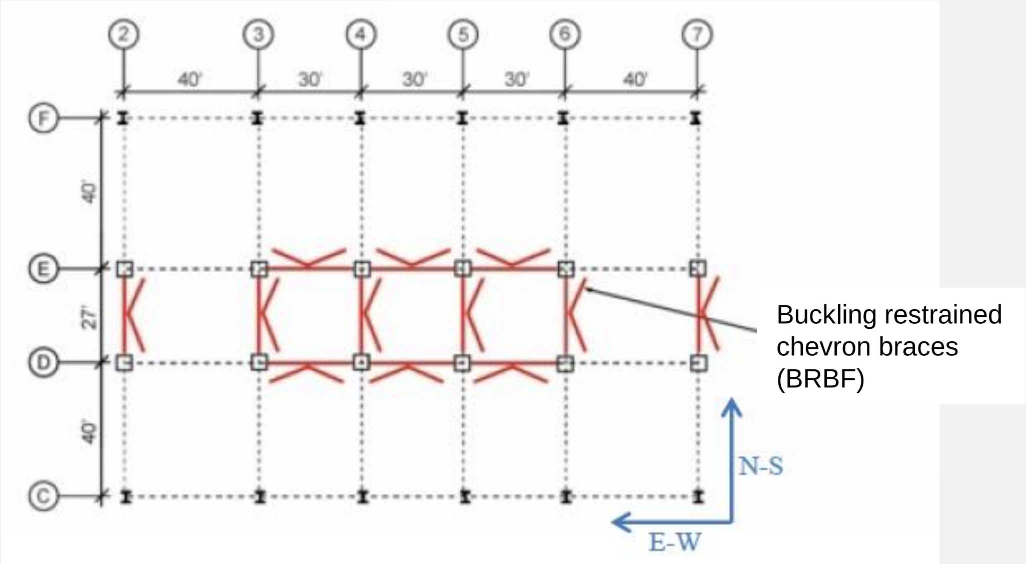
Delay Compensation

$$\tau_k^{(j)} \approx \frac{a_{1k}^{(j)}}{a_{0k}^{(j)}} = 18 \sim 75 \text{ msec}$$

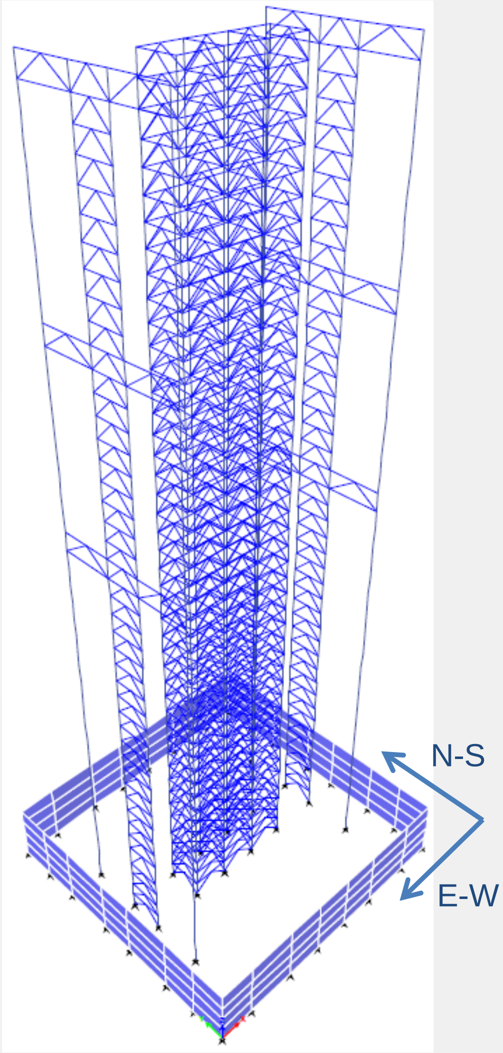


RTHS of a Tall Building

- 40-story (+4 basement) BRBF building in Los Angeles designed by SGH for PEER Tall Building Initiative case studies
- Objectives of study
  - Improve performance using nonlinear fluid viscous dampers with outriggers
  - Assess performance using RTHS
- Extend MKR-α integration algorithm and ATS actuator control to wind natural hazard



Plan for floors that do not include the outriggers. Image courtesy of Dutta and Hamburger (2010)



3-D view of the building. Image courtesy of Dutta and Hamburger (2010)

Ref.: Moehle et al., PEER 2011/05

RTHS Configuration

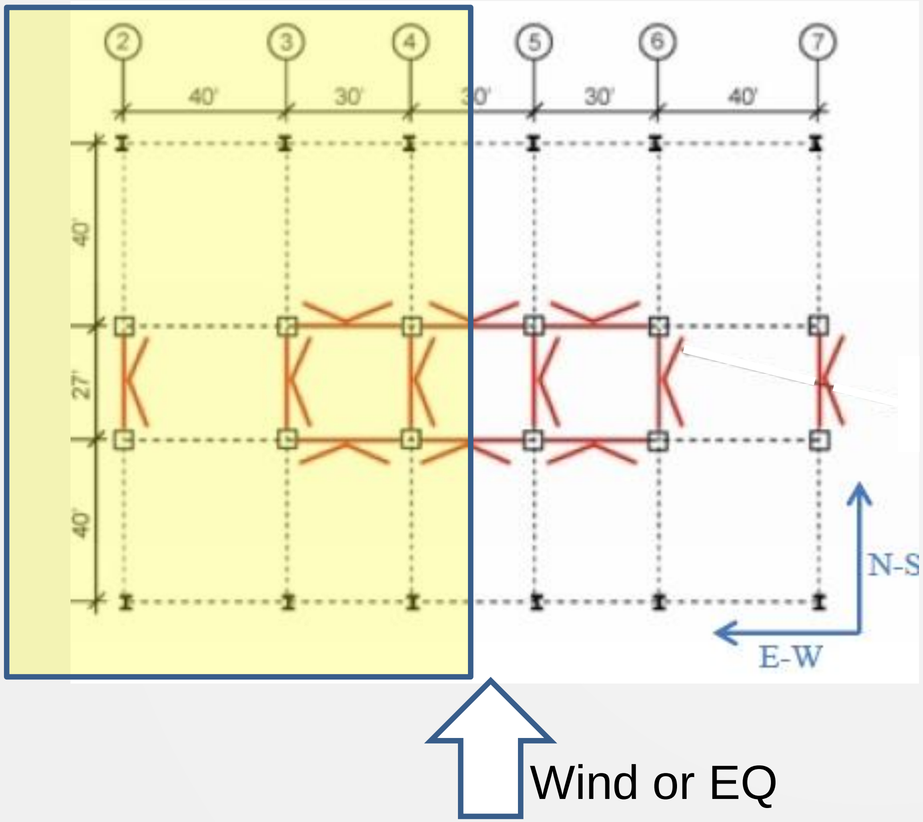
- Use of:
  - Explicit MKR-α Integration Algorithm
  - Explicit Force-based Fiber Element – Analytical Substructure
  - Adaptive Time Series Compensator for Actuator Control

MKR-α parameter and ATS coefficients

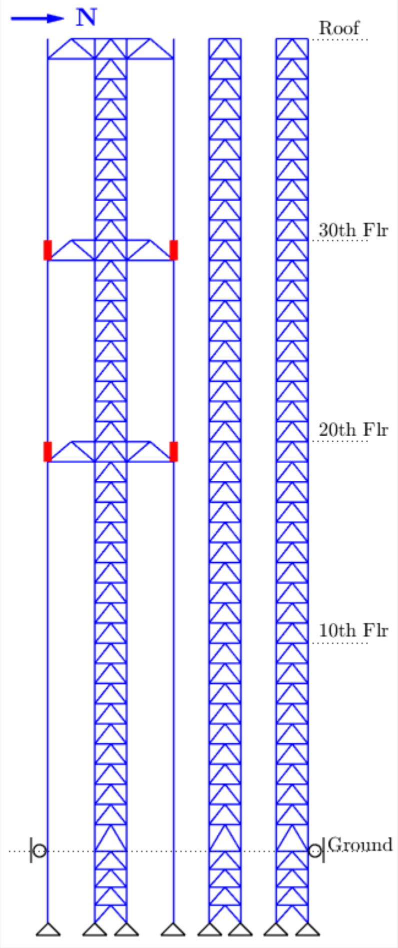
Natural Hazard	Time Step, Δt (sec)	ρ_∞	ATS Coefficients			Comments
			a_{0k}	a_{1k}	a_{2k}	
Wind	$\frac{6}{1024}$	0.866	Fixed	Adaptive	Fixed	Wind: static component with dynamic gusts - 1st mode linear response
EQ	$\frac{6}{1024}$	0.50	Adaptive	Adaptive	Adaptive	EQ: Multi-mode non-linear response

RTHS Configuration

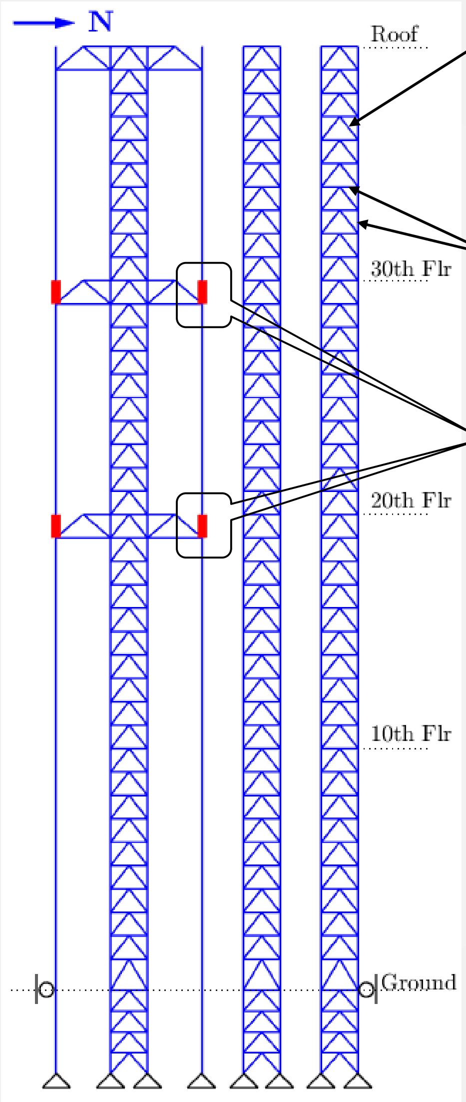
Building Floor Plan



Test Structure Elevation



RTHS Test Structure



BRB: nonlinear truss element with isotropic hardening

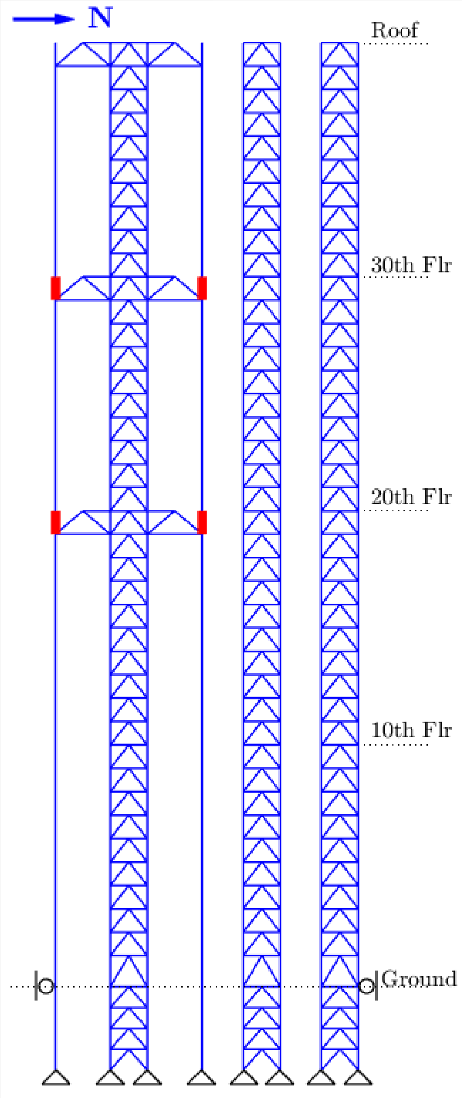
Beams and columns: beam-column fiber element

Nonlinear fluid viscous dampers: Modeled physically

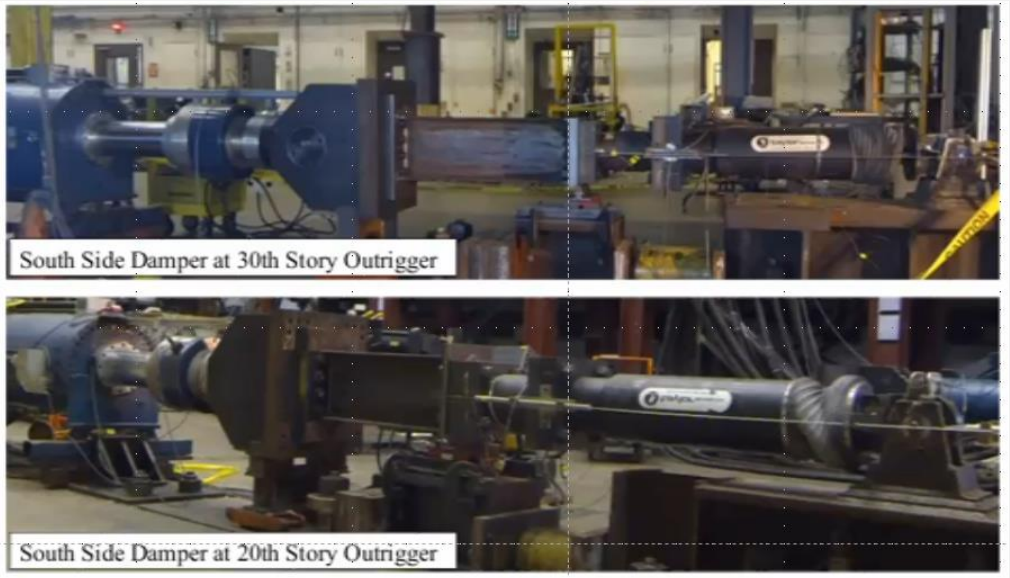
- Mass & inherent damping of building: modeled analytically

RTHS Substructures

Analytical Substructure



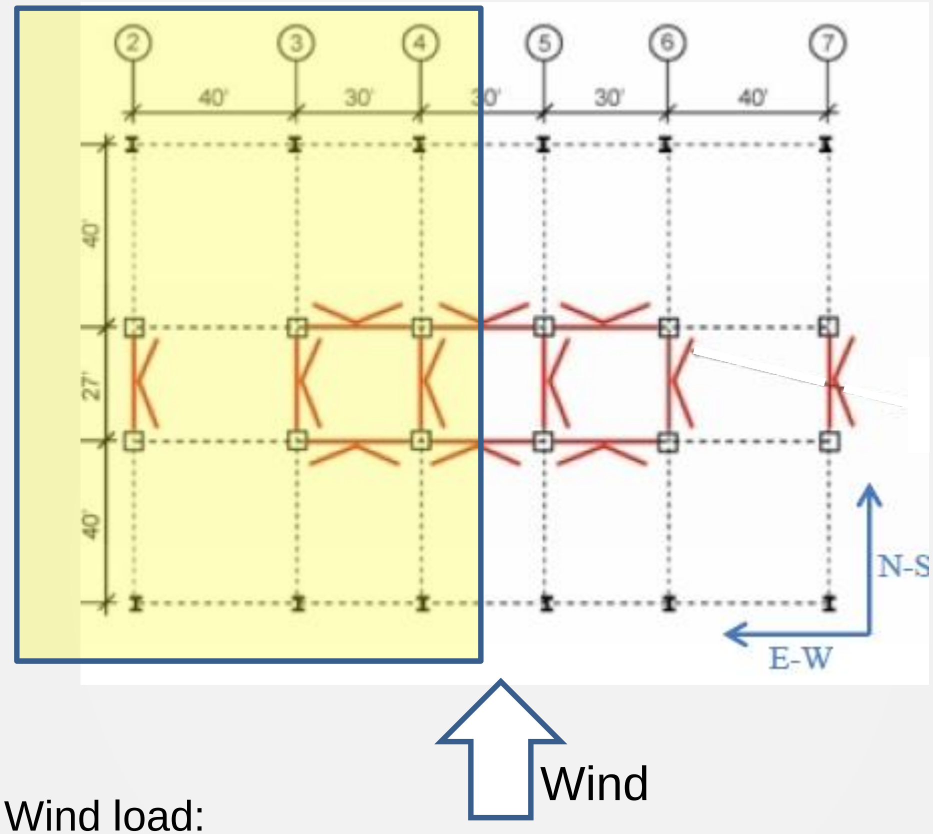
Experimental Substructures



<u>Analytical Substructure Key features</u>:

- P-Δ effects included
- 780 Nodes
- 996 Elements
- 1590 DOFs
- Mass
- Inherent damping of building

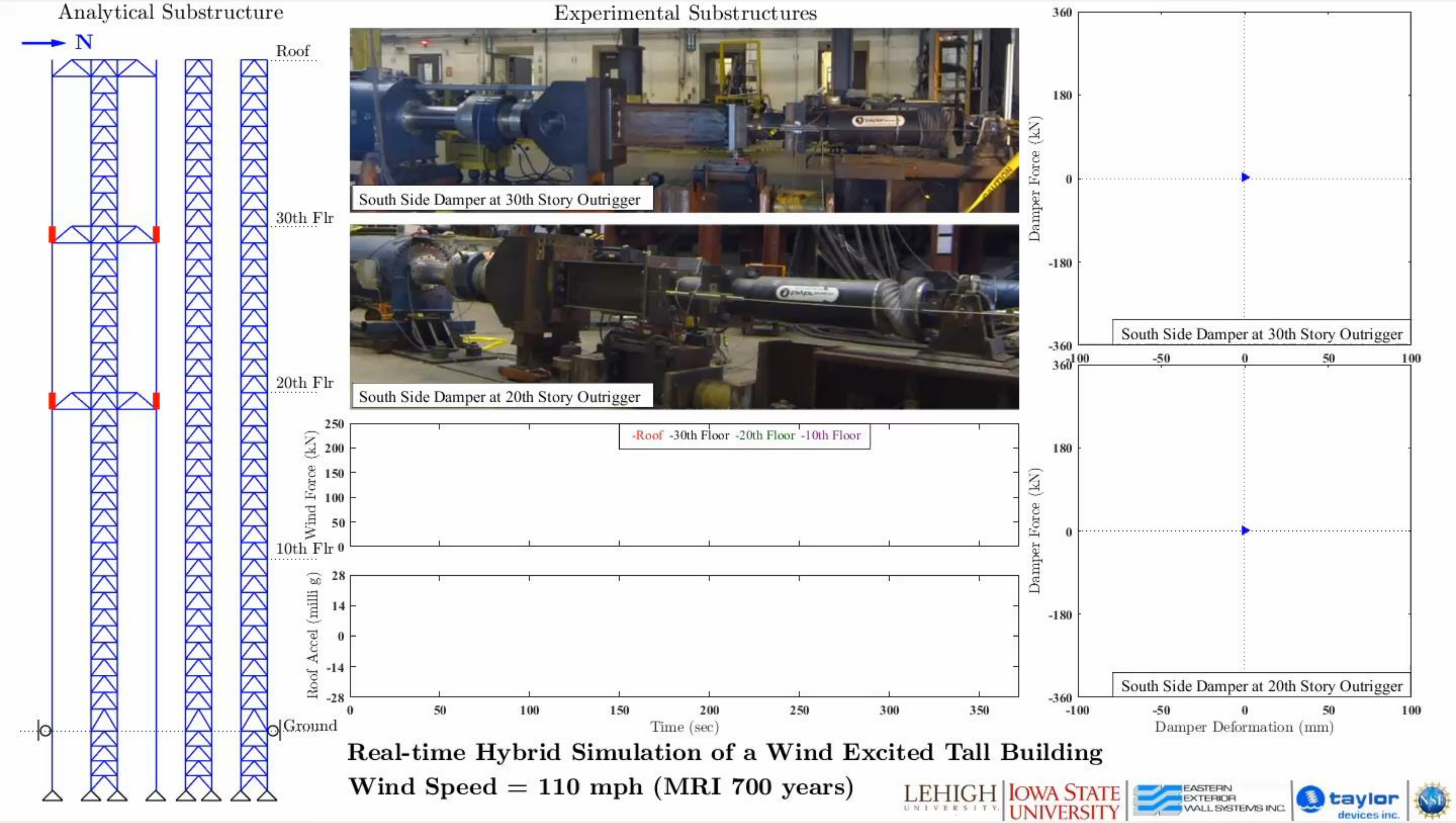
Wind Loading



Wind load:

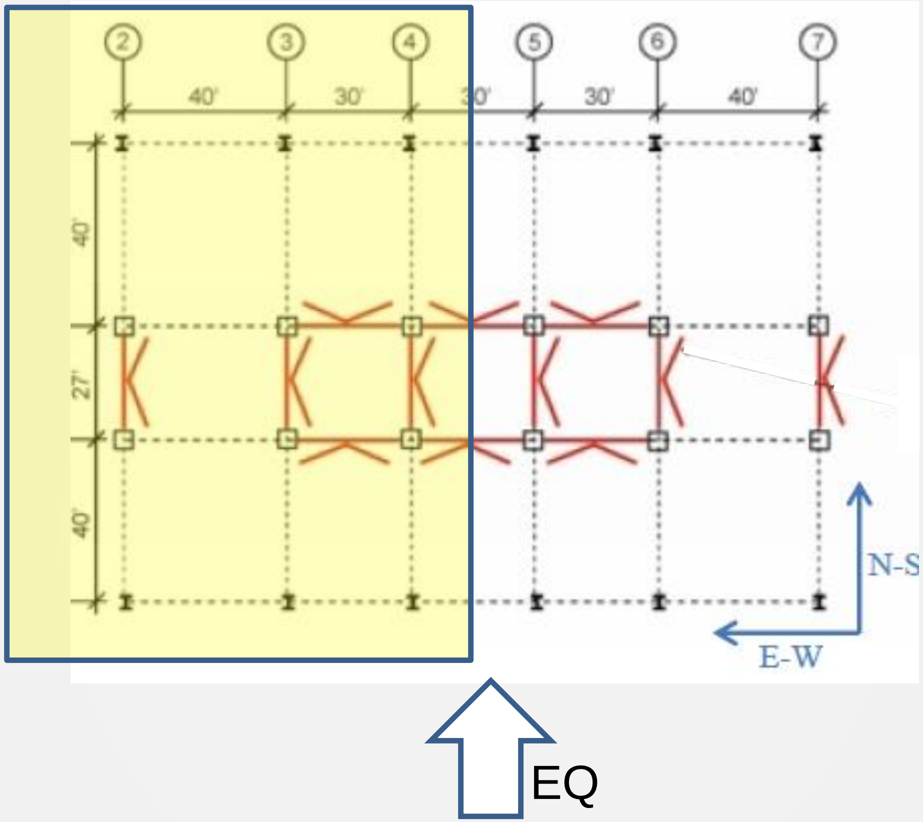
- Tokyo Polytechnic University Wind Tunnel Test database
- Normalized pressure coefficient time histories are converted to full scale forces corresponding to Exposure B and wind speed of 110 mph

Demonstration of a Typical Wind RTHS



LEHIGH UNIVERSITY · ATLSS · RTMD REAL-TIME MULTI-DIRECTIONAL SIMULATION NATURAL HAZARDS ENGINEERING RESEARCH INFRASTRUCTURE · NSF · DESIGNSAFE-CI A NATURAL HAZARDS ENGINEERING COMMUNITY

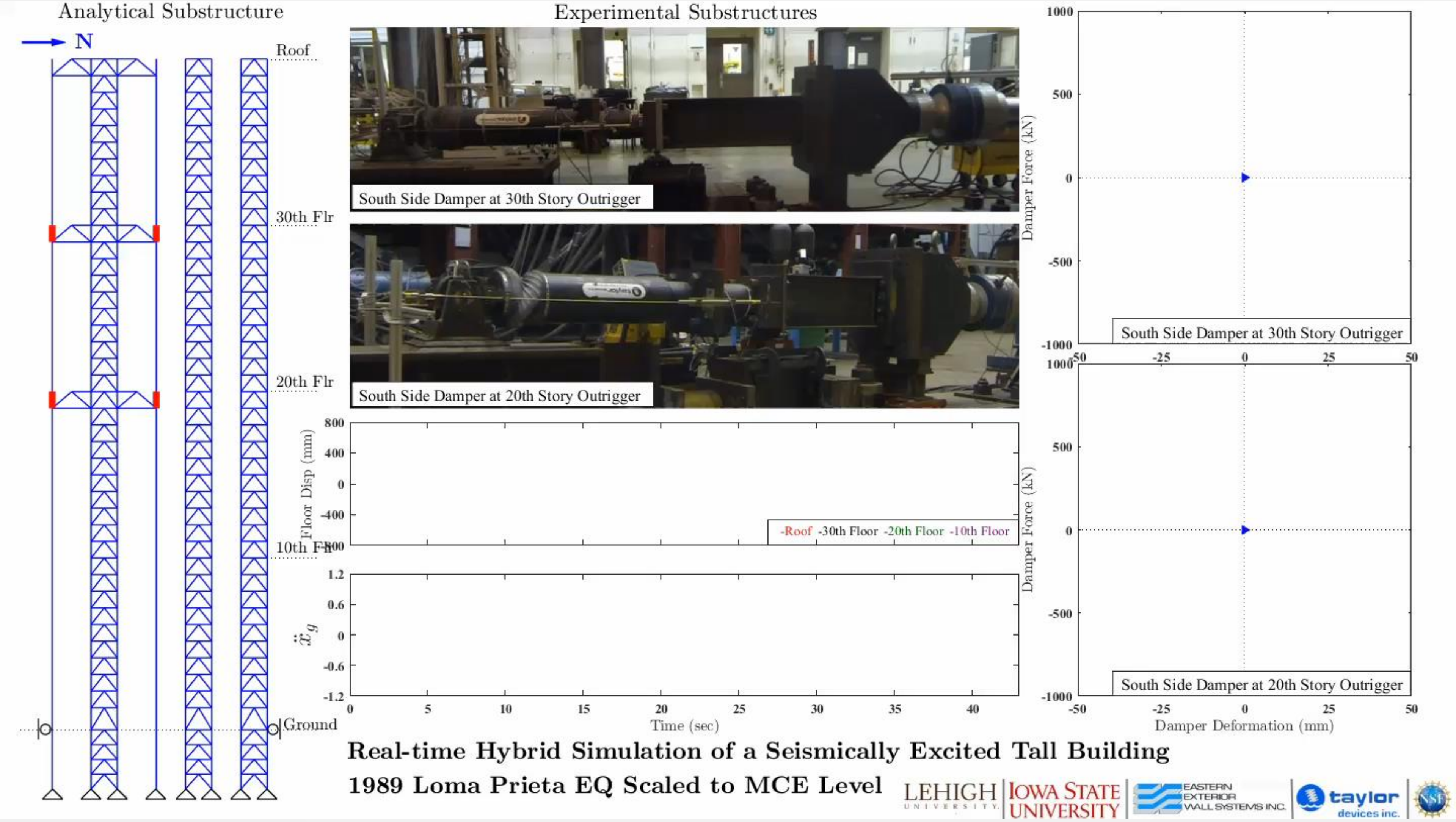
Earthquake Loading



EQ load:

- 1989 Loma Prieta EQ – Saratoga Aloha Ave Station scaled to MCE (2500 year return period) hazard level

Demonstration of a Typical EQ RTHS



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Actuator Control: Typical Wind RTHS

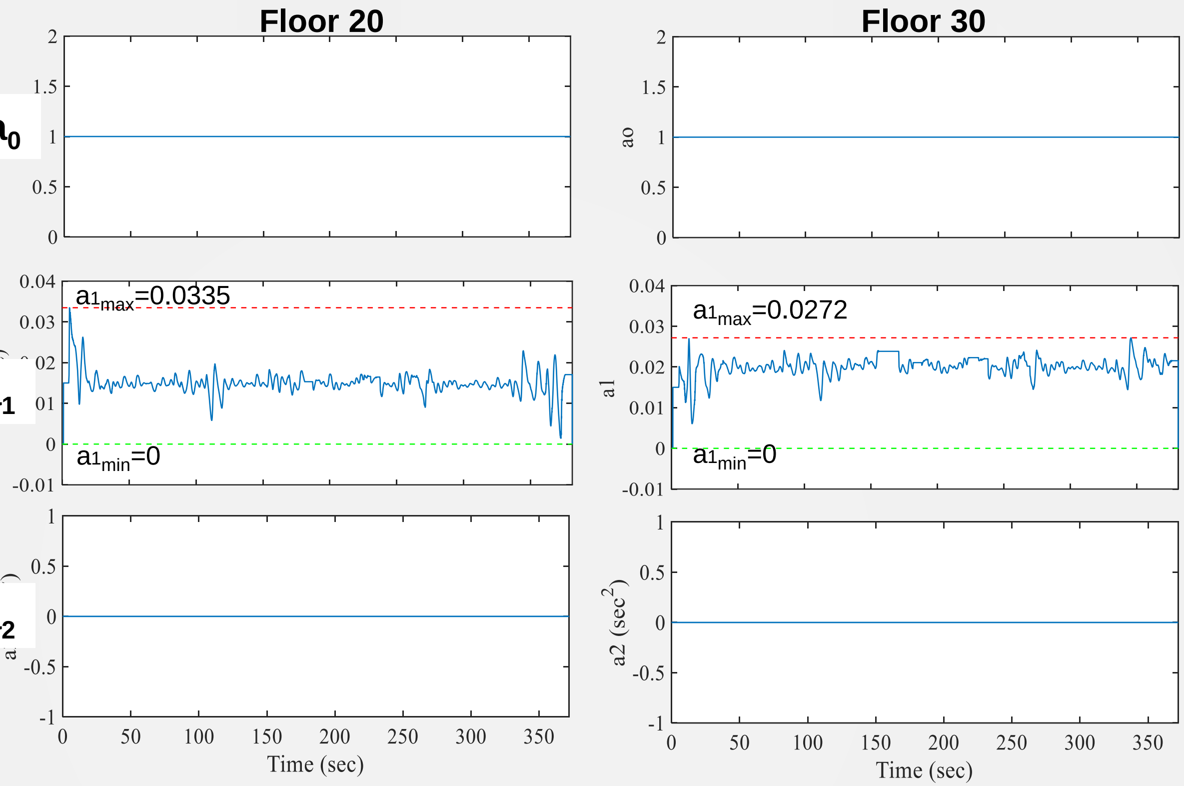
Time History of Adaptive Coefficients

Amplitude Correction

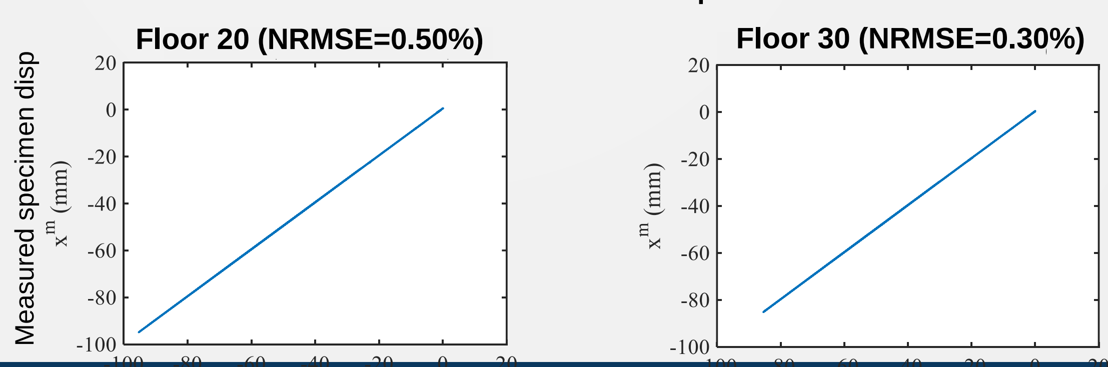
$$A_k^{(j)} \approx \frac{1}{a_{0k}^{(j)}} = 1.0$$

Delay Compensation

$$\tau_k^{(j)} \approx \frac{a_{1k}^{(j)}}{a_{0k}^{(j)}} = 27 \sim 34 \text{ msec}$$



Time variation of ATS parameters



Synchronization subspace plot

Actuator Control: Typical EQ RTHS

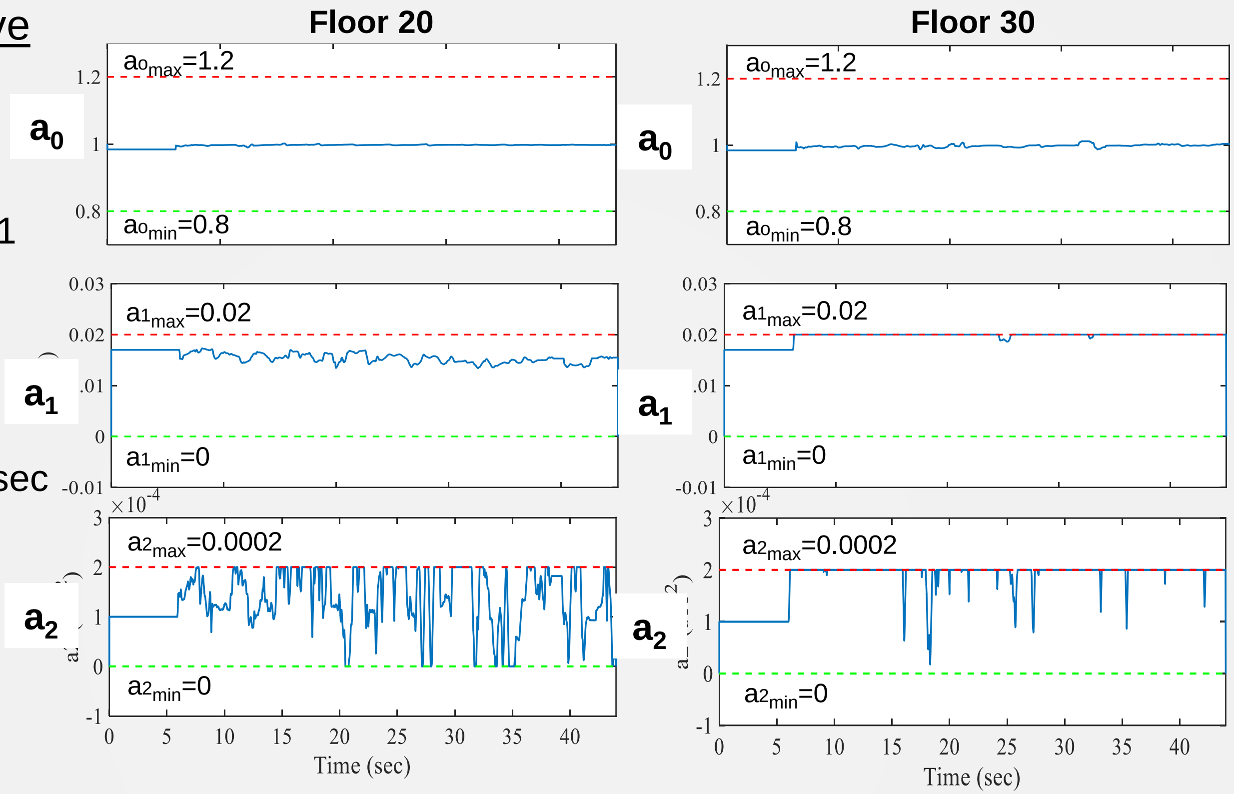
Time History of Adaptive Coefficients

Amplitude Correction

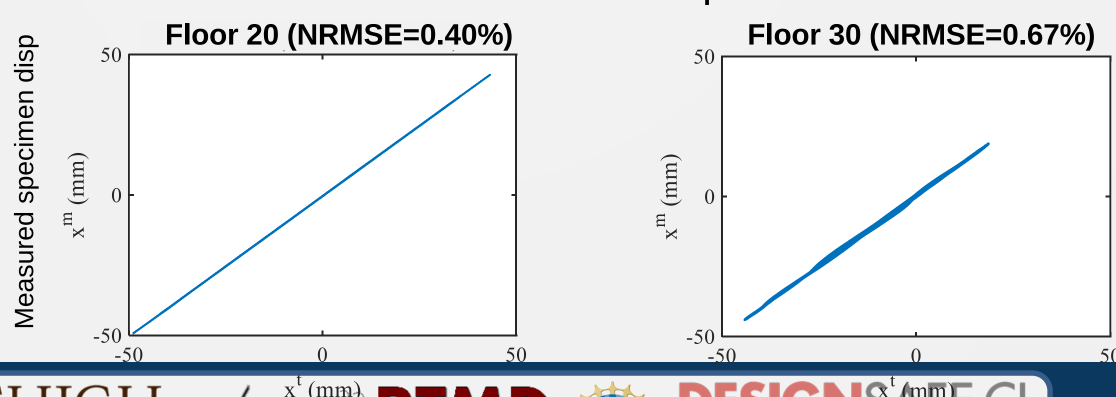
$$A_k^{(j)} \approx \frac{1}{a_{0k}^{(j)}} = 0.99 \sim 1.01$$

Delay Compensation

$$\tau_k^{(j)} \approx \frac{a_{1k}^{(j)}}{a_{0k}^{(j)}} = 15 \sim 20 \text{ msec}$$



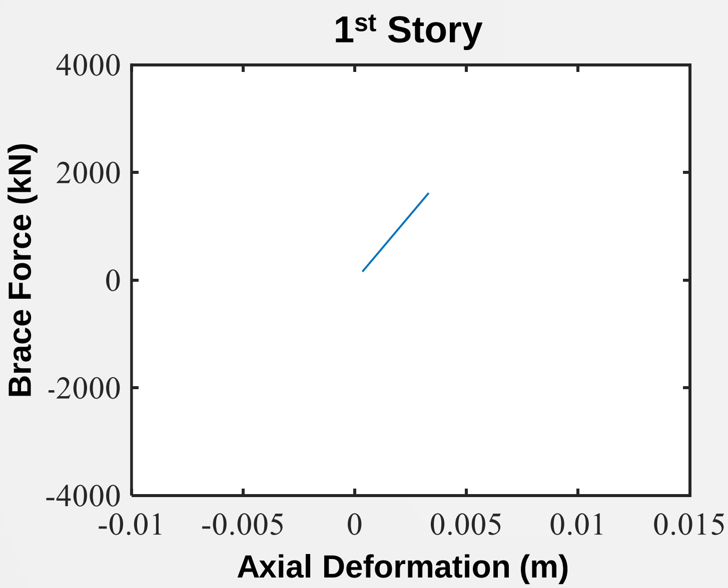
Time variation of ATS parameters



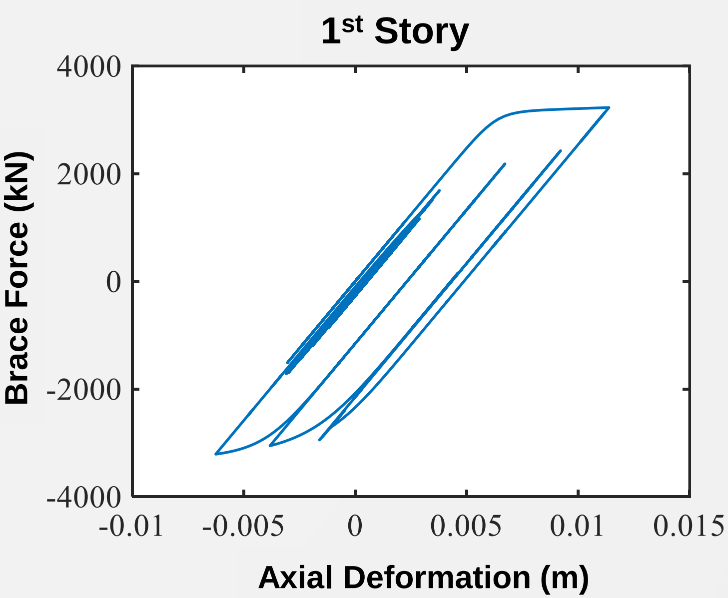
Synchronization subspace plot

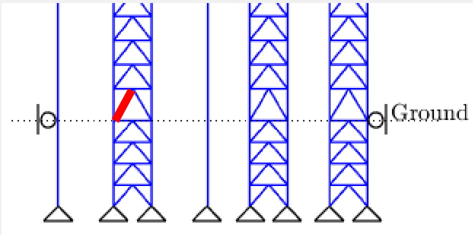
RTHS Analytical Substructure
Buckling Restrained Brace Response

Wind



Earthquake





Summary

- ➢ Reviewed the concept of RTHS
- ➢ NHERI Lehigh Capabilities for conducting RTHS
  - ❑ RTMD integrated control architecture
  - ❑ Various model-based explicit unconditionally stable algorithms with controllable numerical dissipation
  - ❑ Nonlinear computational modeling program: HybridFEM
  - ❑ Multigrid hybrid simulation capabilities
  - ❑ Large capacity hydraulic systems and dynamic actuators
  - ❑ Advanced actuator control: Adaptive Time Series (ATS) Compensator
  - ❑ Example Projects
- ❑ Ongoing developments
  - ❑ Real-time hybrid simulation including soil-structure interaction
  - ❑ Shake table real-time hybrid simulation

Thank you