



UC San Diego  
Structural Engineering  
JACOBS SCHOOL OF ENGINEERING

# Collapse Simulation of Shear-Dominated Reinforced Masonry Wall Systems

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# Most Reinforced Masonry Buildings are Low-rise



*NIST GCR 14-917-31 (NIST 2014)*

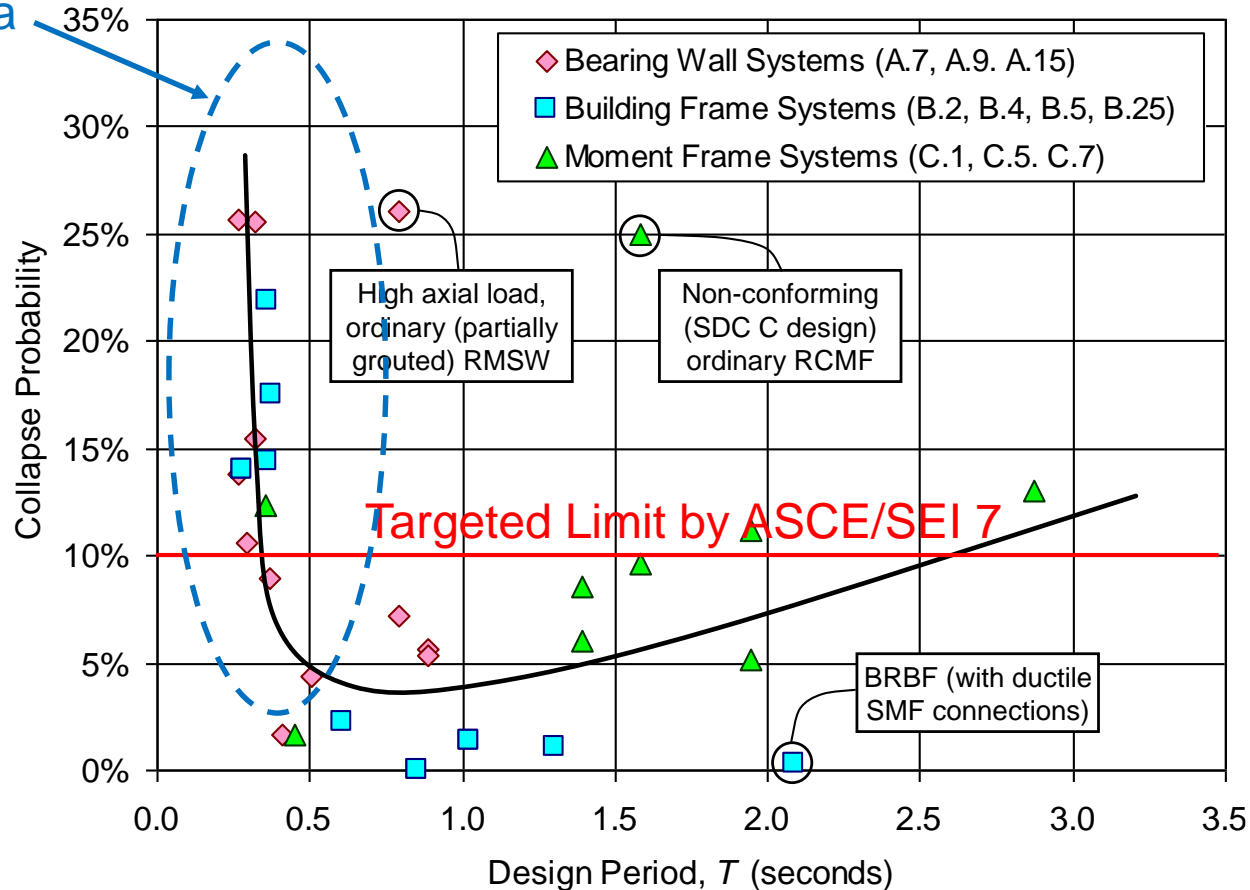


*2011 Christchurch EQ, Ingham & Centeno (2014)*

- Wall elements have low aspect ratios.
- Failure could be dominated by shear even for code-compliant special reinforced masonry shear wall systems.

# Collapse Probability under MCE

Contradicts  
field data

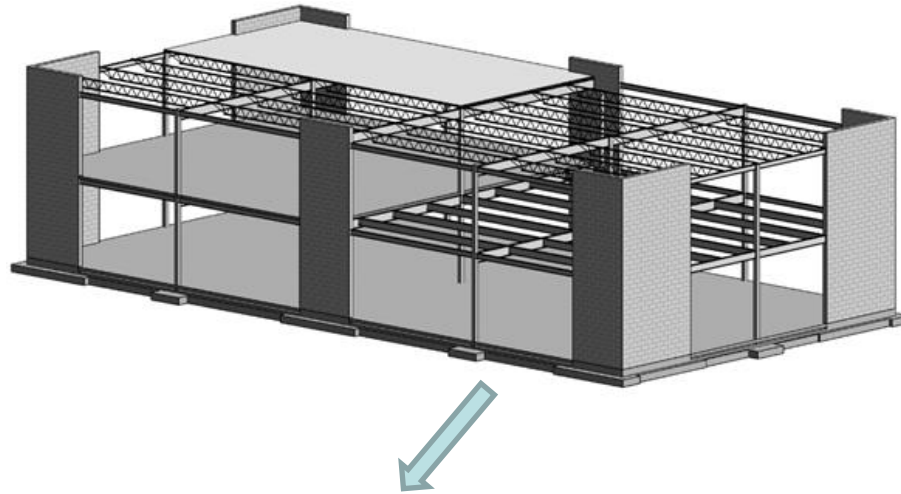


**Results  
based on  
overly  
conservative  
simplified  
models**

*NIST GCR 12-917-20, Tentative Framework for Development of  
Advanced Seismic Design Criteria for New Buildings (2012)*

# ATC 116 Study to Resolve Short-period Building Paradox

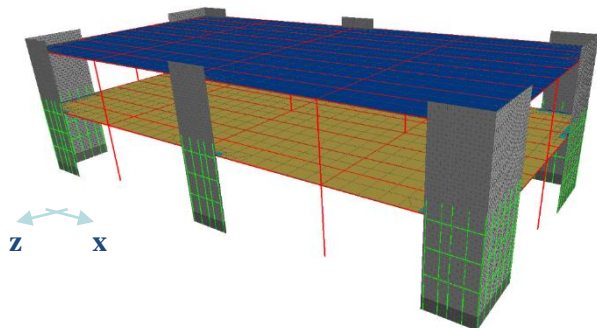
Commercial Reinforced Masonry Building Archetype (6 total)



**FEMA P-695**

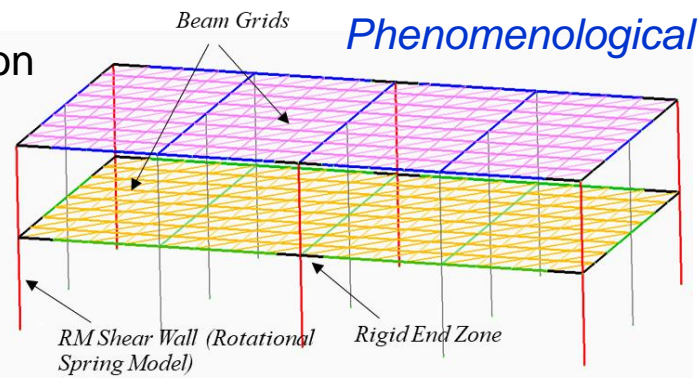
Incremental dynamic analyses to assess collapse probability

Refined finite element model  
(LS-DYNA)

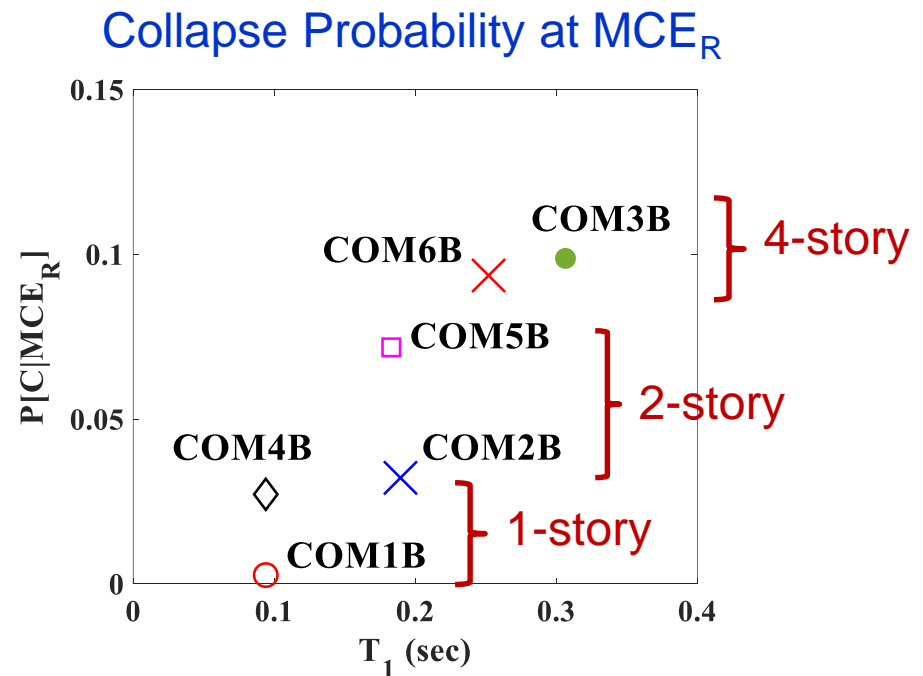
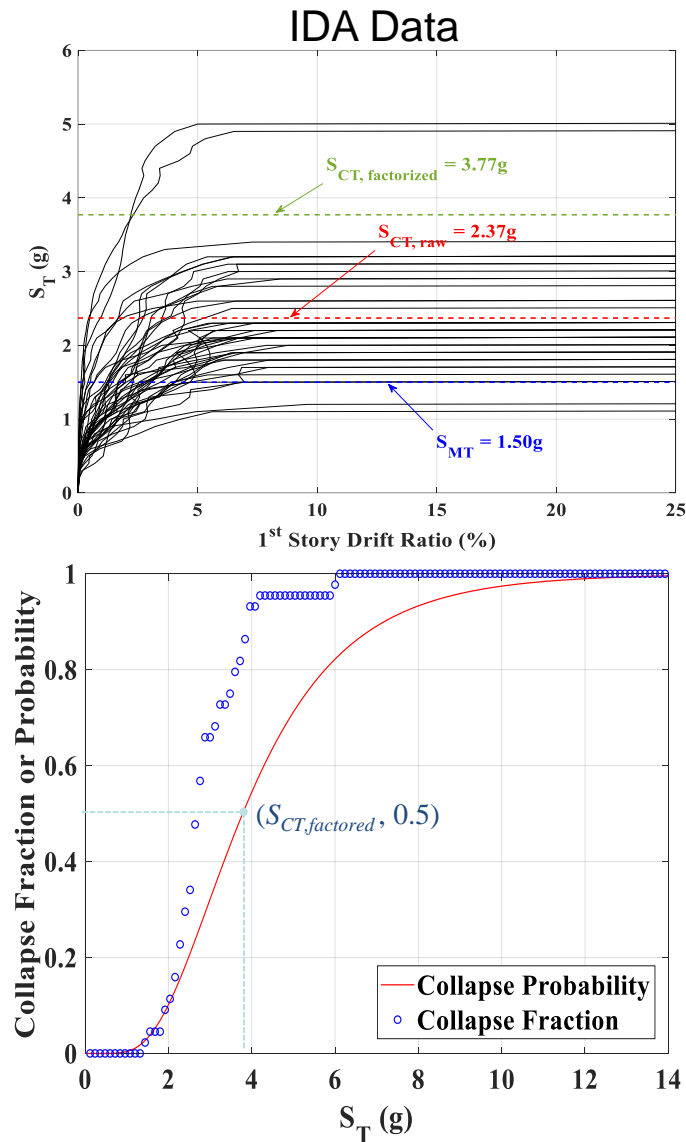


Simplified frame model  
(OpenSEES)

calibration



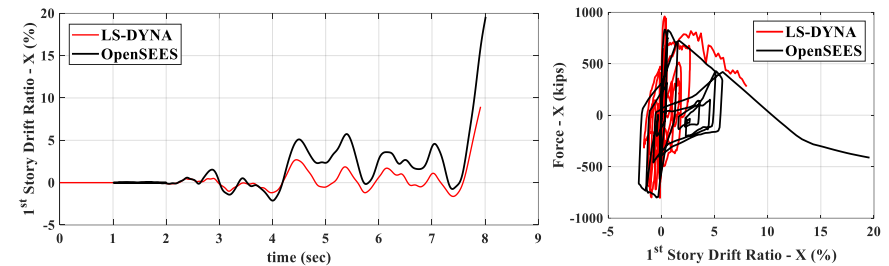
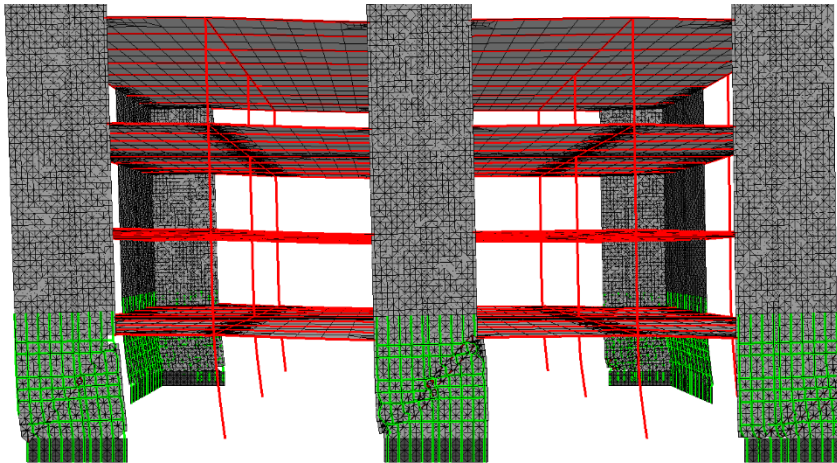
# ATC 116 Study to Resolve Short-period Building Paradox



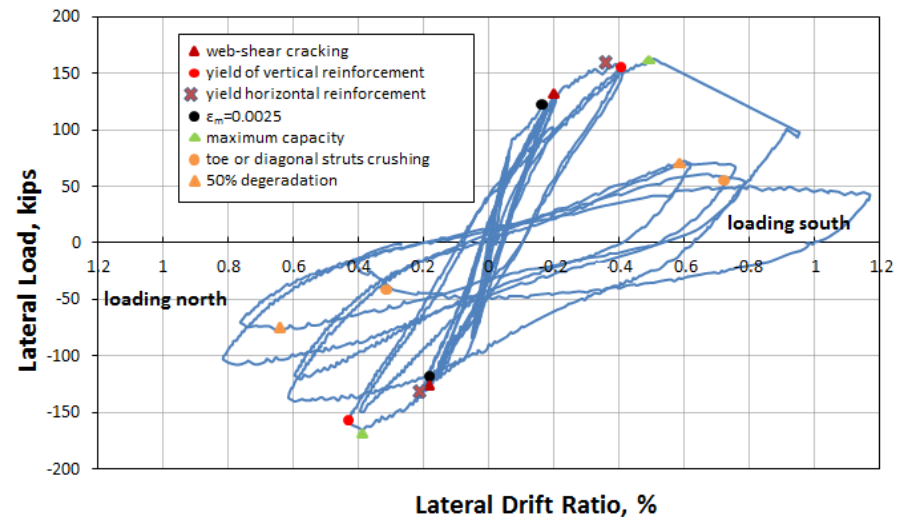


# Are the results trustworthy?

Time = 5.5298



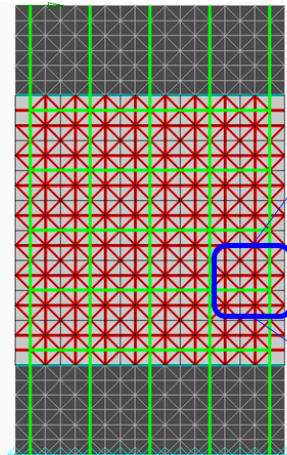
*Quasi-static Test by Ahmadi, Ph.D. Dissertation, UT-Austin (2012)*



# Detailed FE Models

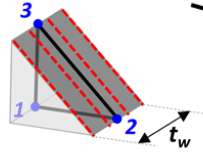
*Koutras, Ph.D. Dissertation, UC San Diego (2019)*

FE discretization of a RM wall



72 in.

**Smeared-crack shell elements**

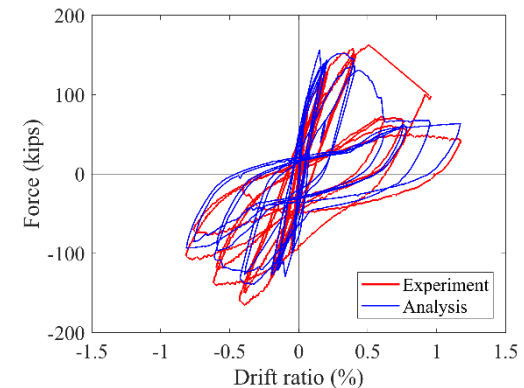
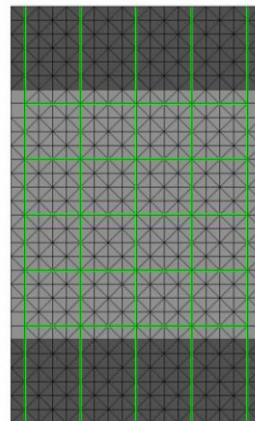


**Zero-thickness cohesive-crack interface elements**

**Fiber-section beam elements for reinforcing bars**

**Bond-slip/Dowel-action interface elements**

Only validated by experimental data with limited drift levels



# Goals of NHERI Project

## Motivation of the Study:

- Lack of experimental data on RM walls tested to collapse.
- Lack of experimental data on shear behavior of flanged RM walls.
- Lack of experimental data on behavior of wall systems at incipient collapse.
- Lack of reliable simplified numerical models to simulate behavior of shear-dominated wall systems.

## Main Objectives:

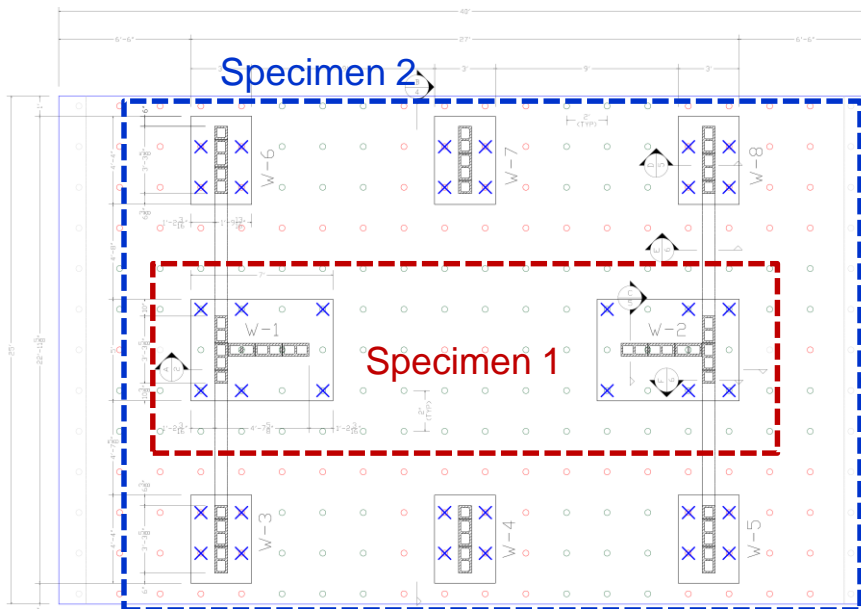
- Validation of computational models for collapse simulation.
- Development of computationally efficient simplified numerical models to analyze shear-dominated wall systems.



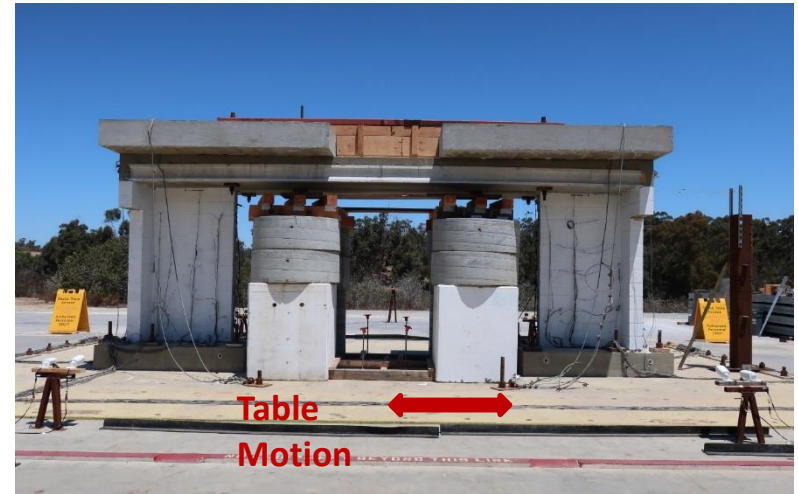
# Additional Goals and Scope

## Additional Goals:

- Quantify the contribution of wall flanges to the shear strength and ductility of walls.
- Quantify the influence of horizontal diaphragms.
- Quantify the influence of orthogonal walls.



Wall Specimen 1



Wall Specimen 2



# Design Considerations

- The two wall specimens were to be tested to near collapse.
- They satisfied the prescriptive design requirements of TMS 402-16 for special RM walls.
- The T-walls in the two specimens had the same design and carried the same gravity load.
- Specimen 1 had the same seismic weight as Specimen 2 but a lower roof weight to have the same gravity load.

## Dynamic Similitude

Specimen 1	
Roof Weight ( $W_1$ )	55 kips
Time Scale Factor ( $\sqrt{W_{1E}/W_{2E}}$ )	0.65
Acceleration Scale Factor ( $W_{2E}/W_{1E}$ )	2.36

Specimen 2	
Roof Weight ( $W_2$ )	135 kips
Time Scale Factor	1
Acceleration Scale Factor	1

$W_{1E}$  and  $W_{2E}$ : expected roof weights

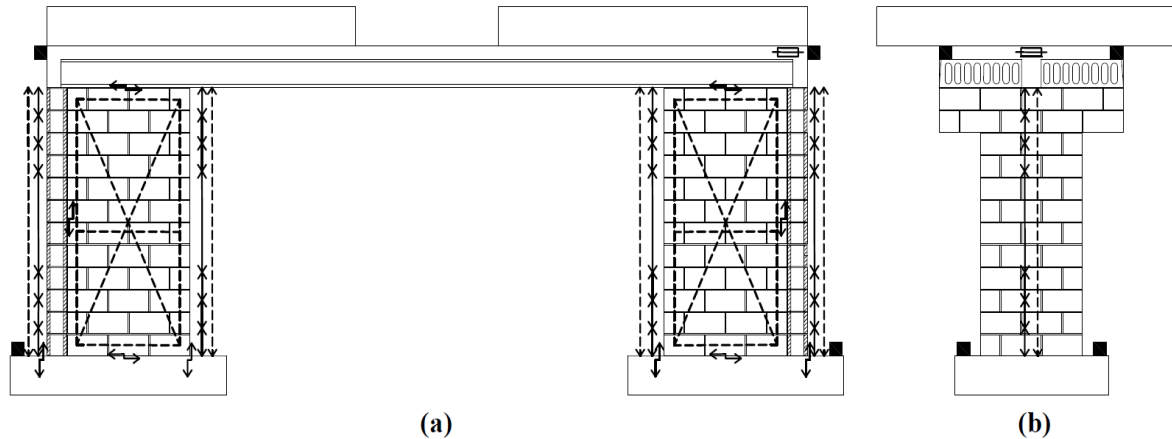
# Construction of Specimen 2

## Masonry Walls Built by Apprentice Masons

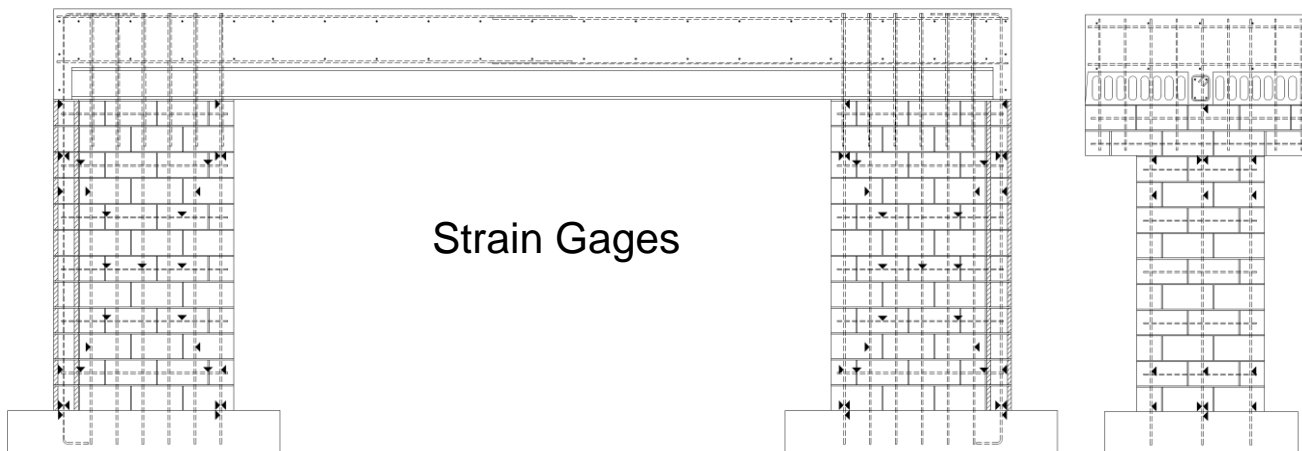


# Instrumentation

## Wall Specimen 1



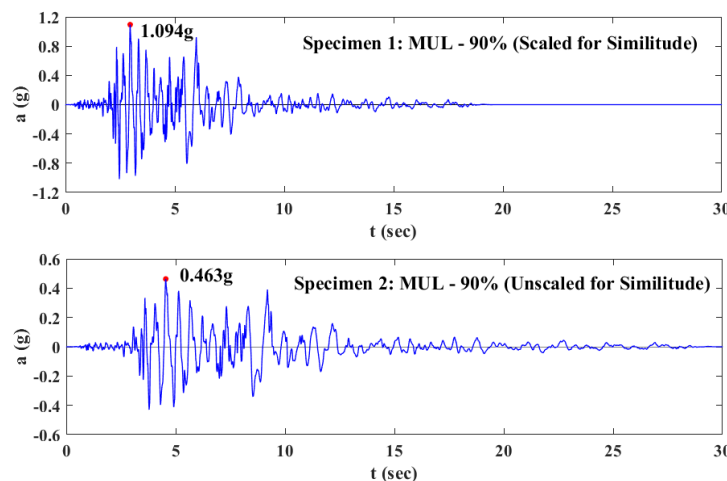
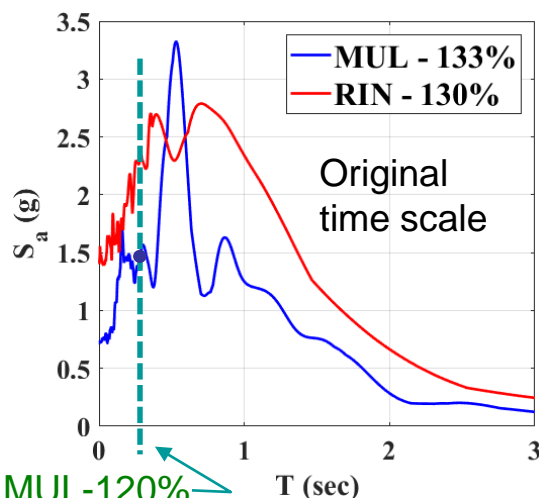
■ Accelerometer    ↔ --- Linear Pot and Stringpot    ↔ Linear Pot for Uplift and Sliding  
— Stringpot measuring horizontal displacement of the roof with respect to the shake-table platen





# Ground Motion Records

## Mulholland and Rinaldi Records, 1994 Northridge Earthquake



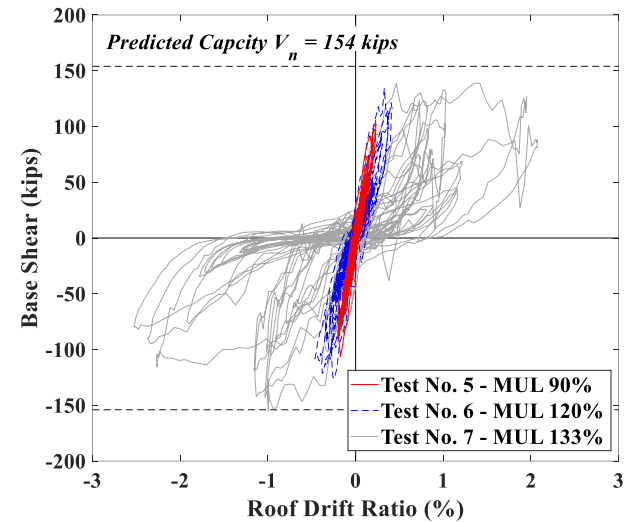
### Scaling Determined by Pretest FE Analyses

Specimen 1			Specimen 2		
Test ID	Input Motion	Period After Test (sec)	Test ID	Input Motion	Period After Test (sec)
	N/A	0.072		N/A	0.090
1	MUL-45%	0.072	1	MUL-45%	0.090
2	MUL-45%	0.074	2	MUL-90%	0.097
3	MUL-90%	0.090	3	MUL-120%	0.121
4	MUL-90%	0.095	4	MUL-90%	0.123
5	MUL-90%	0.107	5	MUL-133%	0.164
6	MUL-120%	0.166	6	MUL-160%	0.328
7	MUL-133%	0.751	7	RIN-130%	-



# Test of Specimen 1

MUL 133%



After MUL 133%



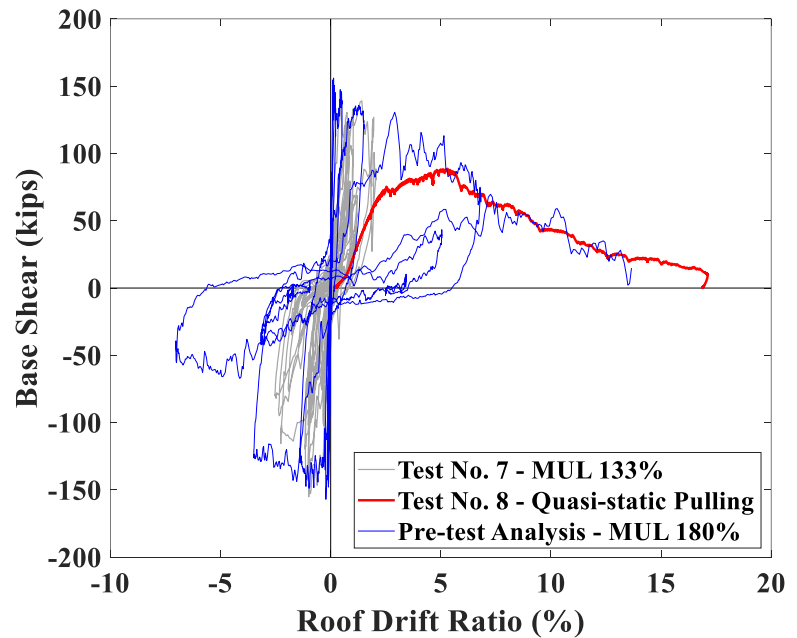
# Test of Specimen 1

Quasi-static Pull Test



FE Model

Comparison w/ Pretest Analysis

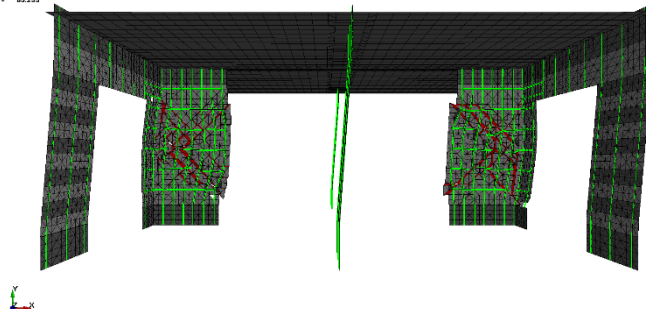


# Test of Specimen 2

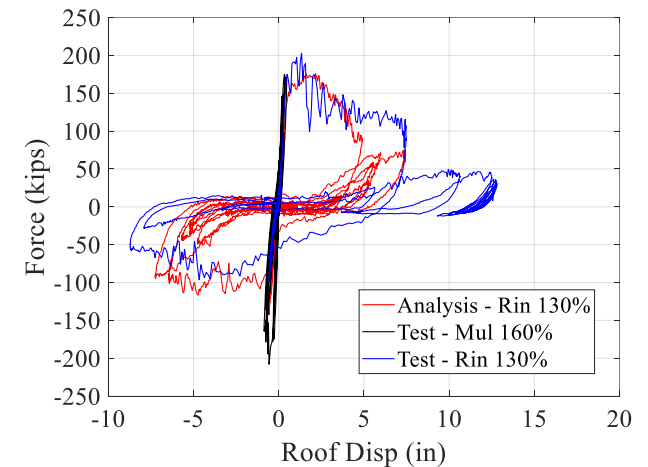
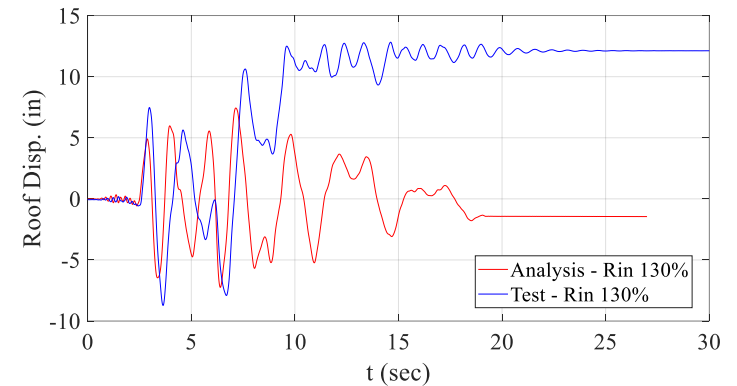
Rinaldi 130%



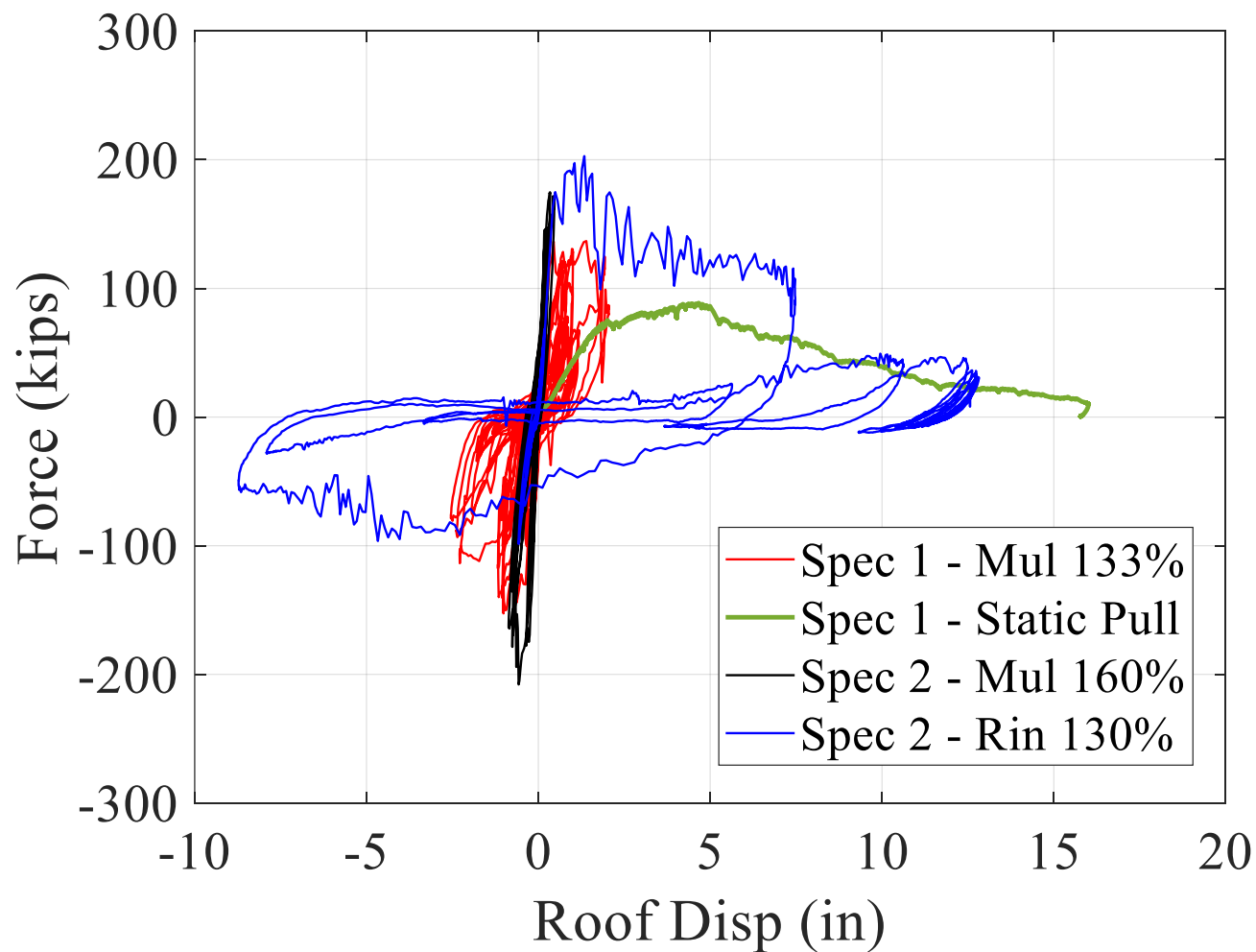
Specimen 2 Pre-test: Mulhol 119%, 133%, and Rinaldi 100%



Comparison w/ Pretest Analysis



# Comparison of Two Specimens



# Additional Work

- Development of more physics-based simplified numerical models to simulate nonlinear shear as well as flexural critical wall behavior.
- Additional numerical studies with detailed FE models to investigate the influence of wall flanges on the shear strength and ductility.



# *Thank you!*

